

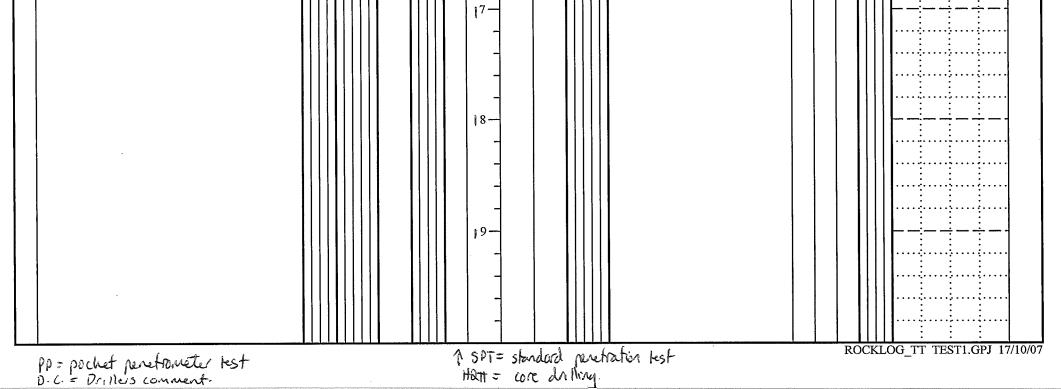
# TONKIN & TAYLOR LTD

## DRILL HOLE LOG

DRILL HOLE No: BH 2, Hole Location:

SHEET 2 OF 2

PROJECT: JAULS POINT - ITER	VIEY DULAN	5	LOCATION	1: MOOLSHENRO	AD QUEENSTALLA	JOB No: 5	380054	
PROJECT:JAULS POINT - ITENLEY DOWNSLOCATION: MODESITED ROAD, QUEENSTOWN JOB NO: 880054CO-ORDINATES MNDRILL TYPE:NDR 650HOLE STARTED: 10/10/2007								
mE			DATUM:			HOLE FINISHED: 10/10/2007		
DIRECTION: H人 °			R.L. GROUND: m		DRILLED BY:	DRILLED BY: MUNEILL DRILLING		
ANGLE FROM HORIZ .: 90°, vertical			R.L. COLL	· · · · · · · · · · · · · · · · · · ·		LOGGED BY: ۲۰۰۰ CHECKED:		
DESCRIPTION OF CORE			S.P.T-Logs		<u>093</u>	· · · · · · · · · · · · · · · · · · ·		
ROCK OR SOIL TYPE, WEATHERING,		sing S	ں ب		T JOINTS, BEDDING,	۲. C		
LARDNESS, STRENGTH, COLOUR,	4GY ERING	0 / UC (MPa) LOSS & CAS	(WBO		ND SHEARED MS PE, SHAPE, ROUGHNESS,	RQD (%) WATER (ILL WATE OSS (%)	(m)	
foliation, mineralogy, texture, etc);	ROCK MEATHERING ROCK STRENGTH	PT LOAD / UCS TEST (MPa) CORE LOSS /LIFT (%) ), CORE & CASIN	TEST SYMBOL DEPTH (m) GRAPHIC LOG	CRUSHED A CRUSHED A ZONES/SEA LDC trues LDC trues DEFECT TYI DEFECT TYI DEFECT TYI	PE, SHAPE, ROUGHNESS, H	RQD (%) WATER DRILL WATER LOSS (%)	SOR O CON	
HARDNESS, STRENGTH, COLOUR, LITHOLOGICAL FEATURES (bedding, cement, foliation, mineralogy, texture, etc);	× v			E			SPT RESULTS °	
	RR RR RR RR RR RR RR RR RR RR RR RR RR				SPT LOGS	សិចិស	o 8	
					SPI LOGS	- 55 - 75		
p gravel. 9.5-10.0 = no sords and PP< 50kPa., SILT, dry, 31. plus, s very shiff pp= 50-150kPa, occusion Dougland free grand, becoming pu	t/P-		XO X	Co.4 1095				
B PP< 50kPa., SILT, dry, Sl. plut, S B Very Shiff pp=50-150kPa, occusion	nd	LOT.						
J rounded fine grand, becoming and claypy 10.0-11.0m (0.3. 100)	v~e	Ŧ						
	÷.		- 11 - R.K.F.		Contact.			
Core Run => 11-0 - 12.4m			× × × ×		11-0m - brown		SPTC: 11.0m	
11.0-11.50 pp=50-150kla givey bown SI with mixor clay + occasional sandy leng u1.5-12.00 pp=350kla, sitty SAND(f- with mixor clay, sl. plast. thinky knin 12.0-12.40 pp=50-100kla sitty SAND	ej	Sp		moist.	SILT, sh plastic, firm to stiff.		6, 7, 9, 10, 11, 7	
11.5-12. Ompp= 350kha, sitty SAND(f-	() .fed		- × × ×		,, p		Λ= 50 fer 265mm	
with minor clay, st. prast. Thinky which 12-D-12-1, PD=50-100hla sitty SAND	3 NOA		- X X					
plustic, non cohesine, laminted. (NO 1055)		€						
		/						
core ends at 12.4 m				aliania hara alianza di Colte 2000 Alianza	204 m - 10 sample.		<u>. SPT @ 12.4m</u>	
				Sample			10, 21, 19 1=50 61.70m	
U.C SAND			13 - \ /	10 00				
51.AC1.AL			š - X	lare				
4				SPTC	3.5m - browngrey		SPT@ 13.5m	
D.C. = GRAVEL		SPT		silty S	AND with gravel		Cardinal and a second sec	
				Sand = (	f-c) gravel = (fm)		5,12,15,8 <u>N=50 fer 150</u>	
				10	,			
			= - V					
				lone				
D.C. = GRAVEL								
				SPTC	5.0m - brown		SPTEISOM	
Y		SPT			SILT, stiff, moist,		3,5,6,7,7,7	
END OF DRILLHOLE B. 15.45M.				Pleasion	al gravels rounded	<del>╶</del> ╞╼╼ <del>╞</del> ╴╊┾┾╋	1n= 27	
				(+-i).	-			
			16-					



Appendix D: CPT Logs

## **CPT ANALYSIS NOTES**

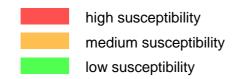
#### Soil Type

Interpretation using chart of Robertson & Campanella (1983). This is a simple but well proven interpretation using cone tip resistance ( $q_c$ ) and friction ratio ( $f_R$ ) only. No normalisation for overburden stress is applied. Cone tip resistance measured with the piezocone is corrected with measured pore pressure ( $u_c$ ).



#### **Liquefaction Screening**

The purpose of the screening is to highlight susceptible soils, that is sand and siltsand in a relatively loose condition. This is not a full liquefaction risk assessment which requires knowledge of the particular earthquake risk at a site and additional analysis. The screening is based on the chart of Shibata and Teparaksa (1988).



High susceptibility is here defined as requiring a shear stress ratio of 0.2 to cause liquefaction with  $D_{50}$  for sands assumed to be 0.25 mm and for silty sands to be 0.05 mm.

Medium susceptibility is here defined as requiring a shear stress ratio of 0.4 to cause liquefaction with  $D_{50}$  for sands assumed to be 0.25 mm and for silty sands to be 0.05 mm.

Low susceptibility is all other cases.

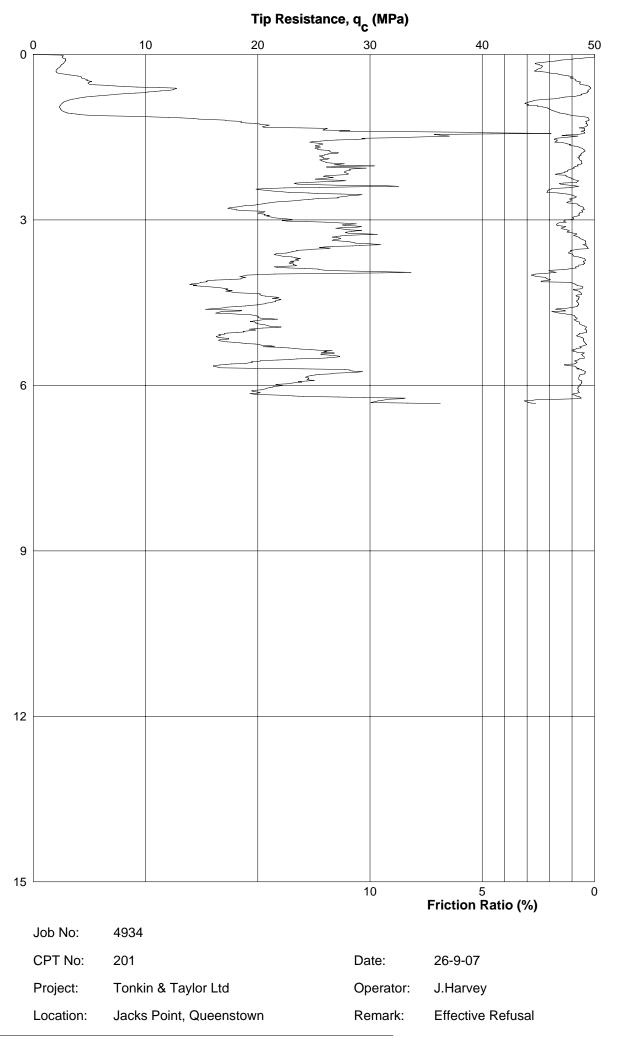
#### Relative Density (D<sub>R</sub>)

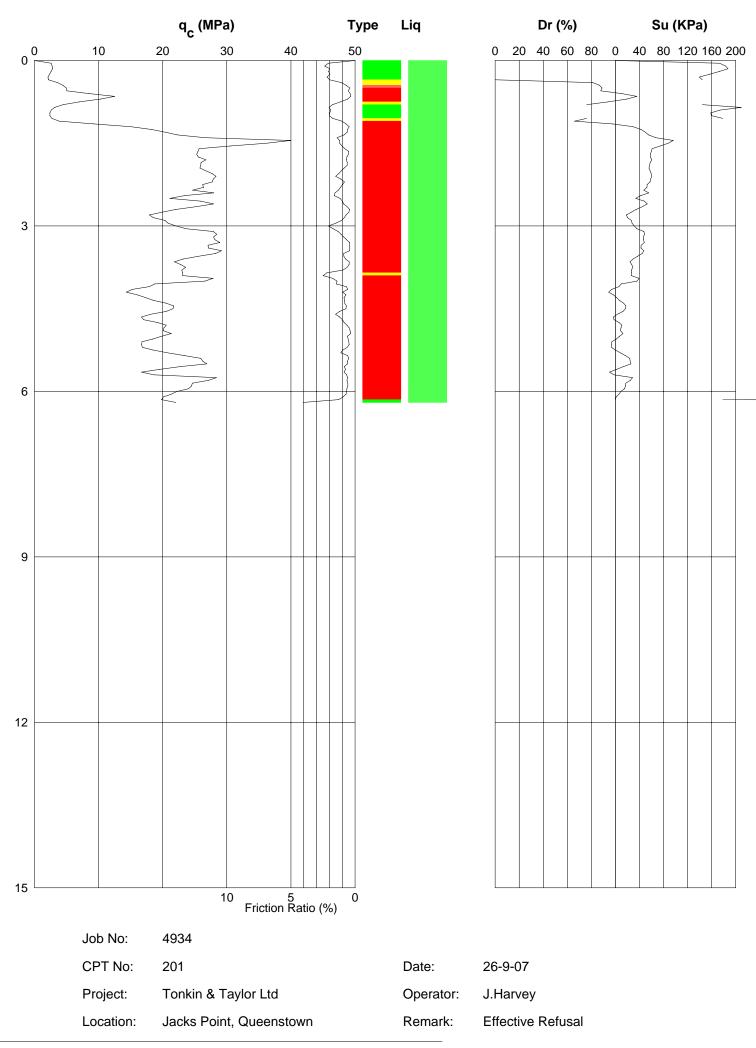
Based on the method of Baldi et. al. (1986) from data on normally consolidated sand.

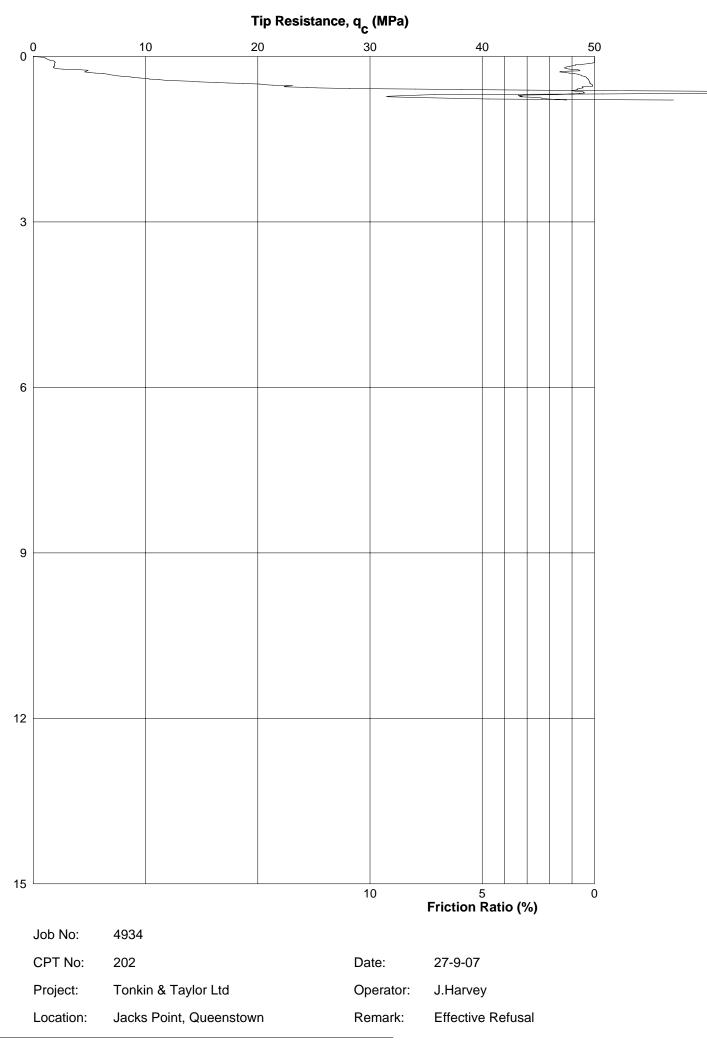
#### Undrained Shear Strength (S<sub>U</sub>)

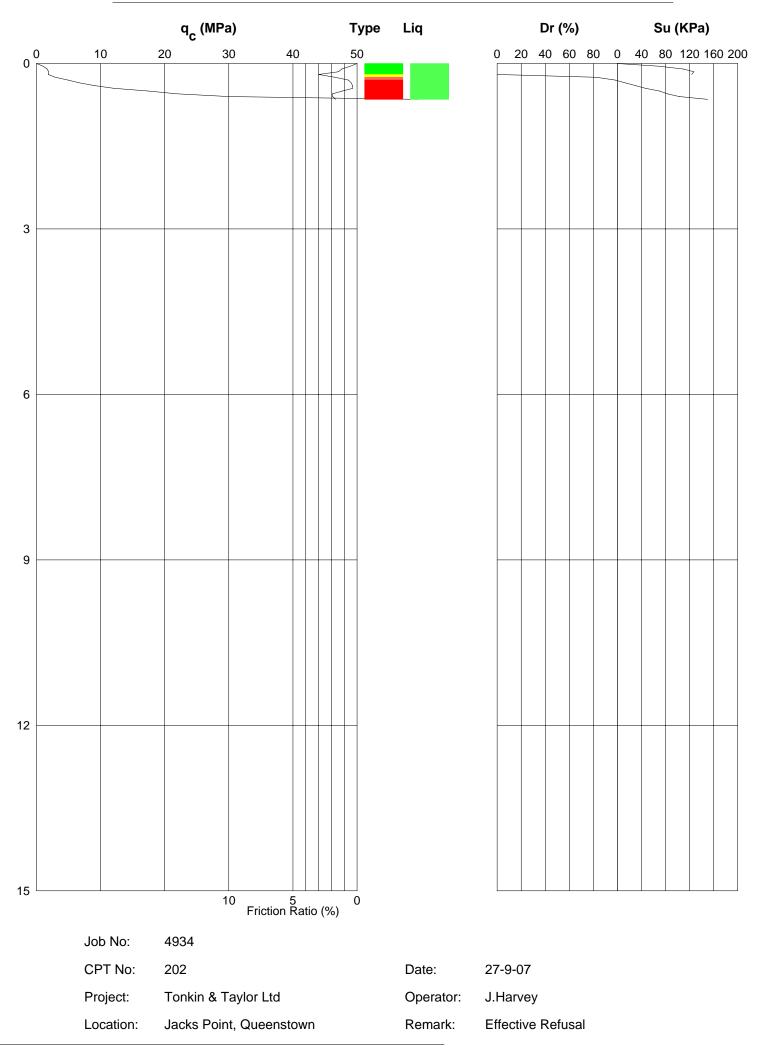
Derived from the bearing capacity equation using  $S_U = (q_C - \sigma_{VO})/15$ .

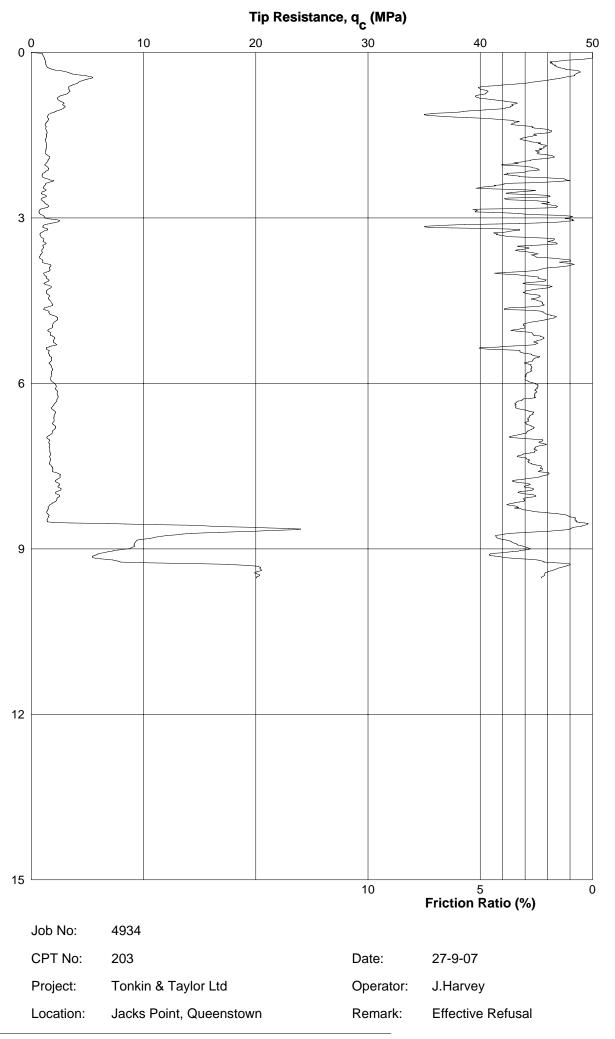


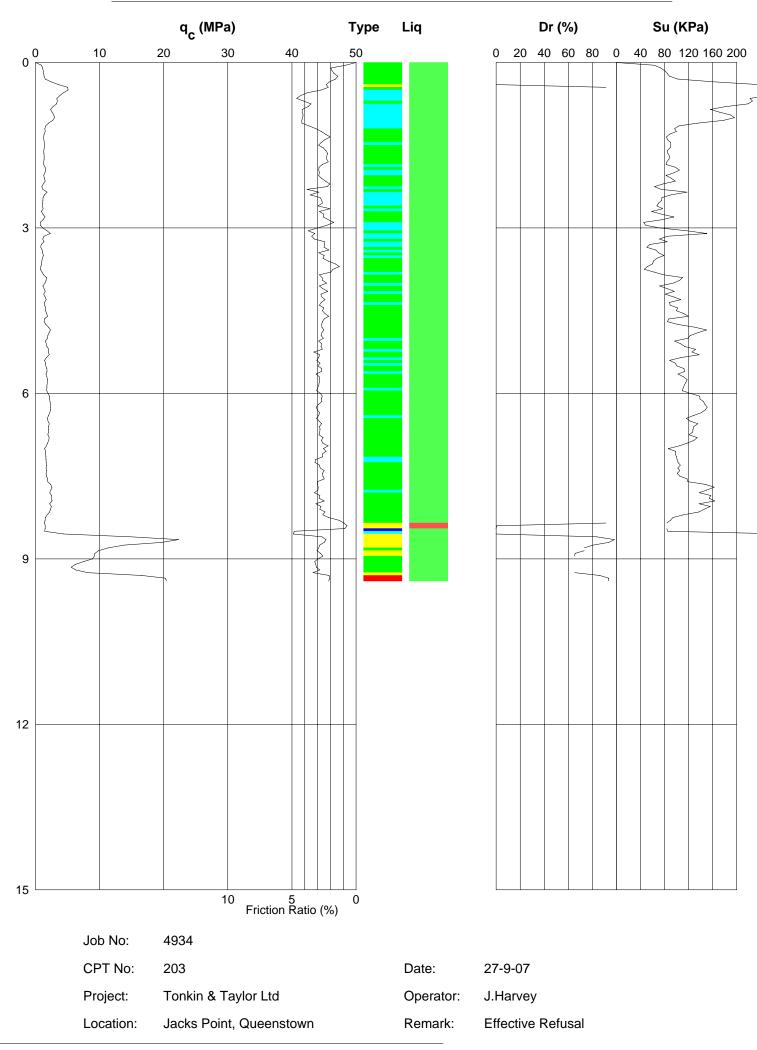


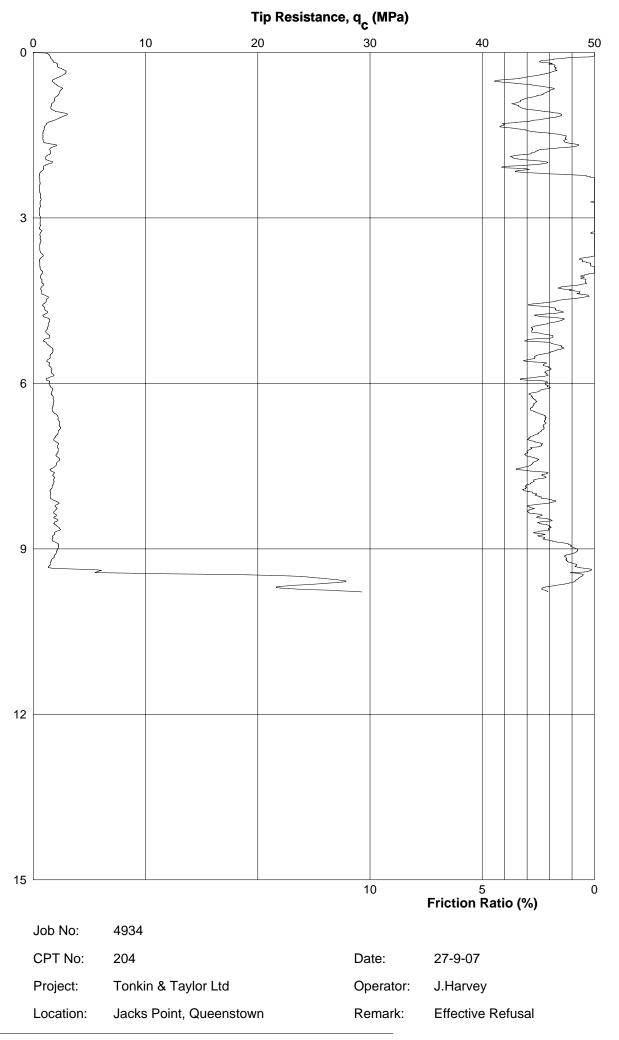




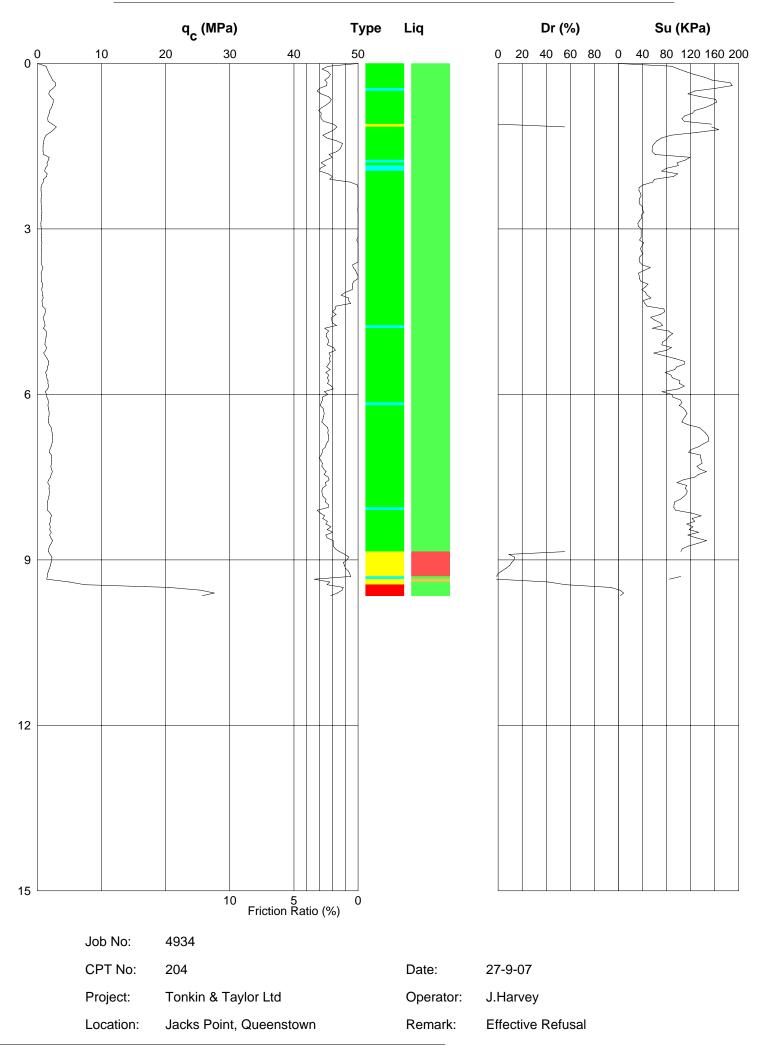




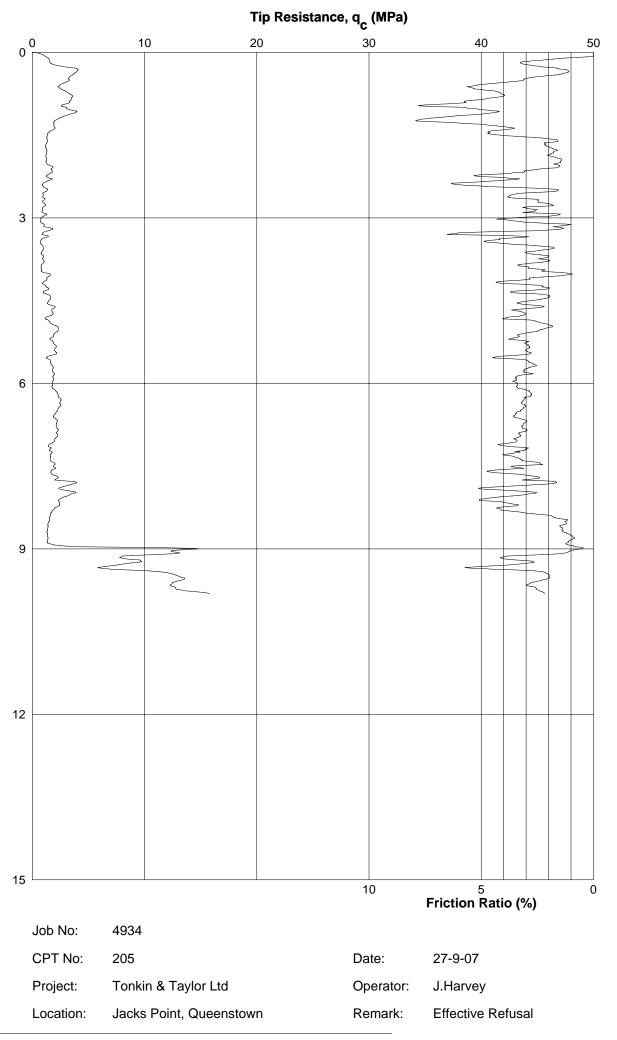


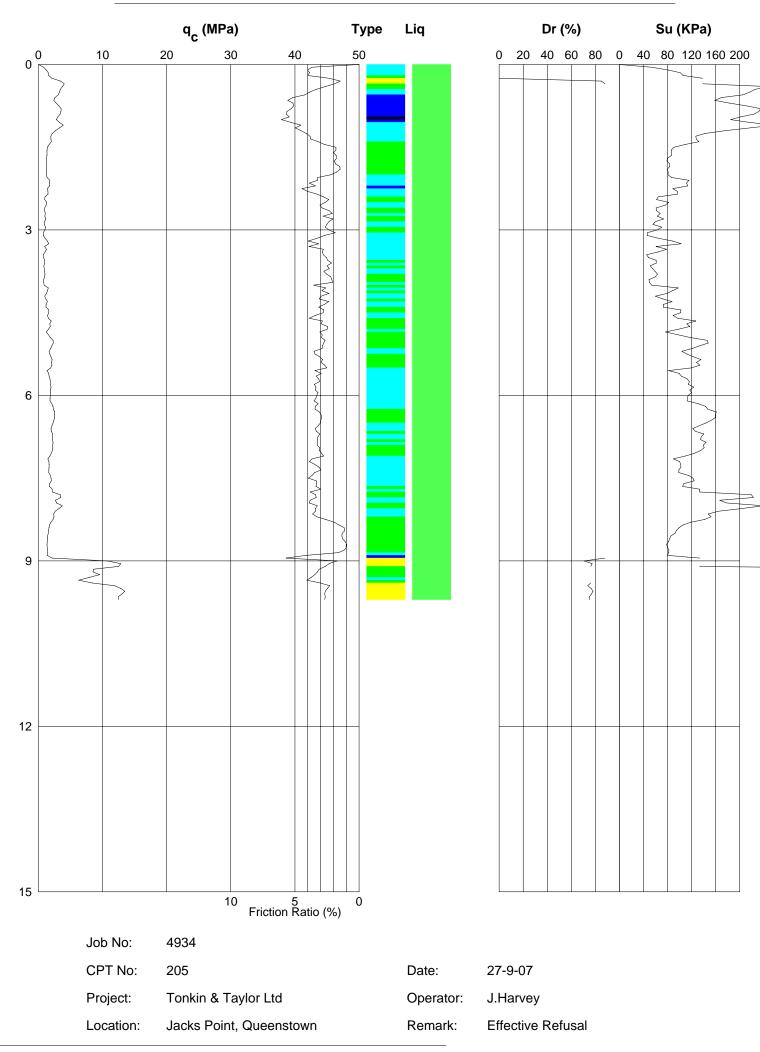


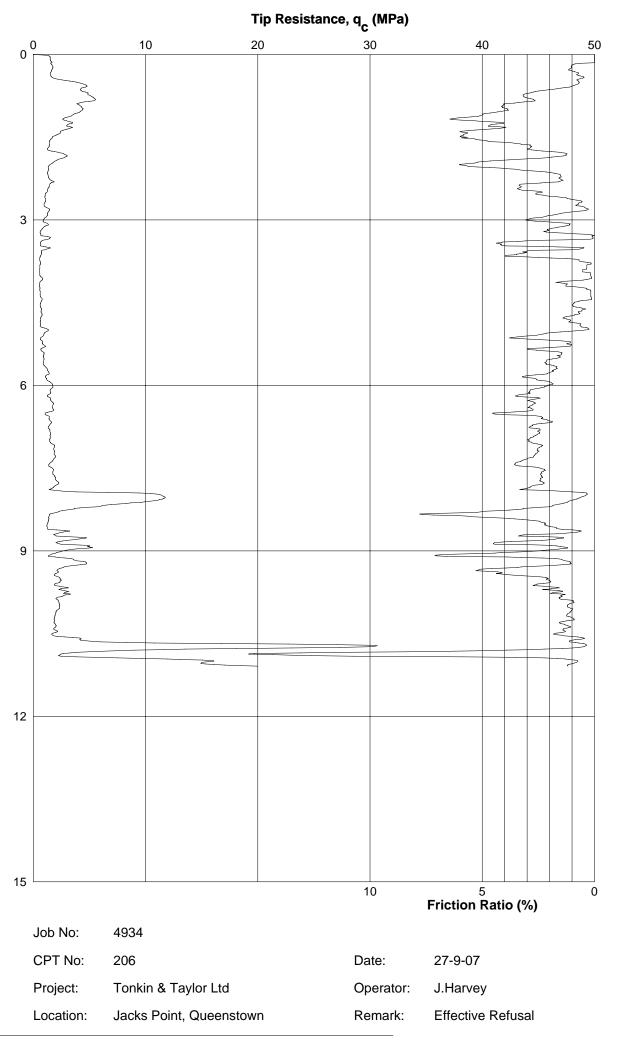
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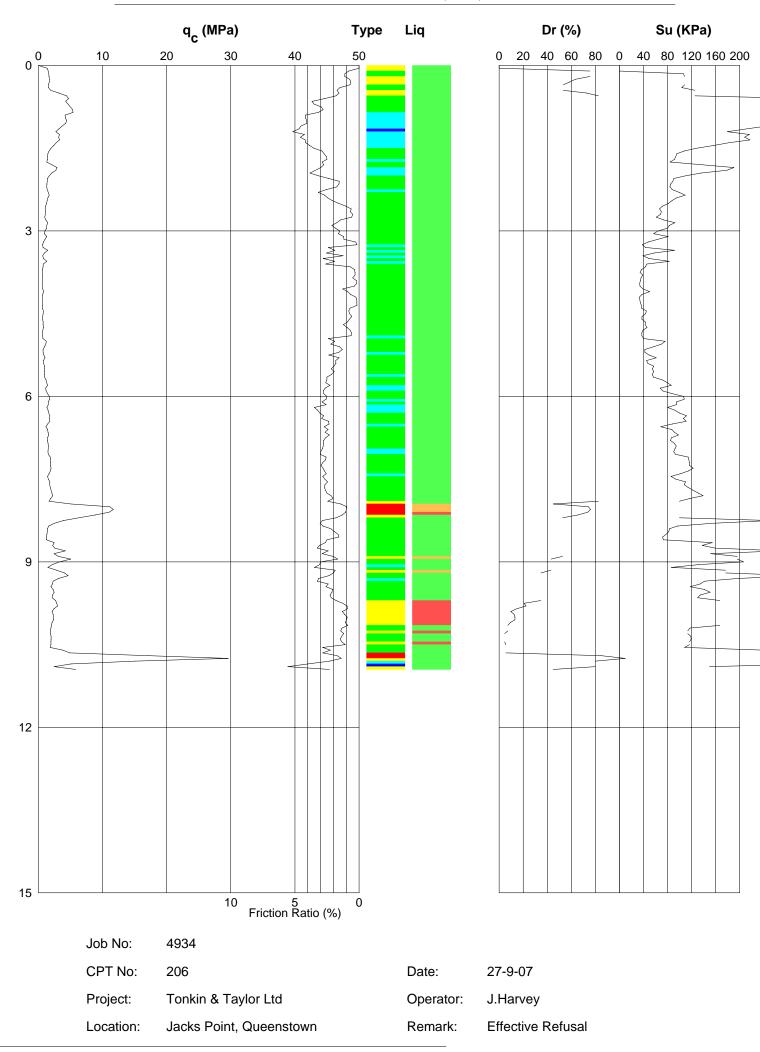


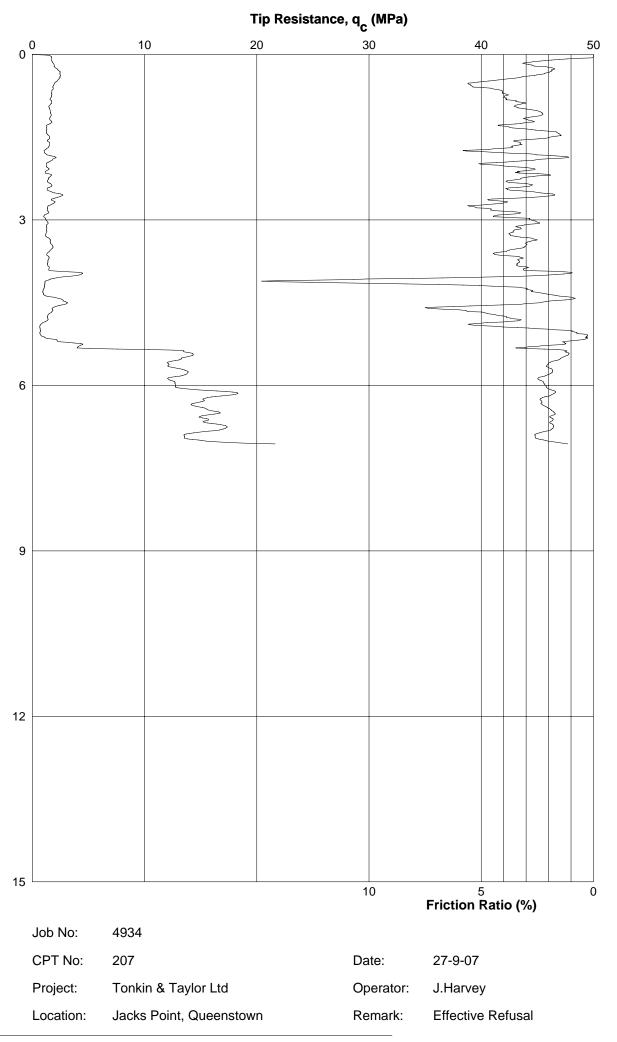
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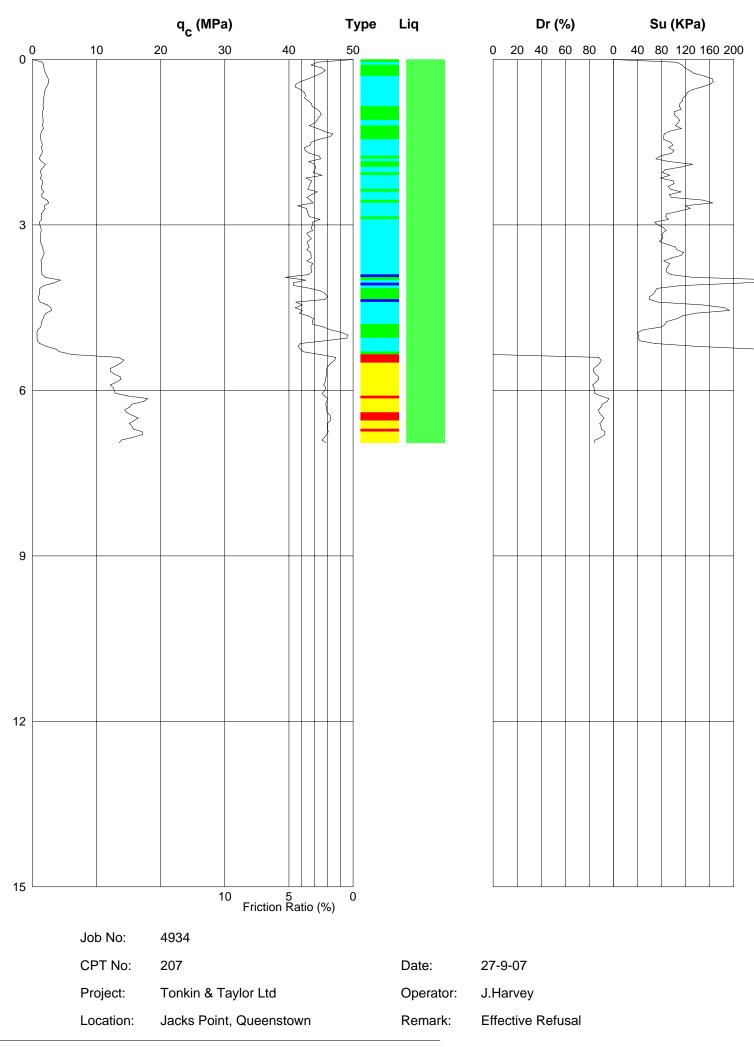


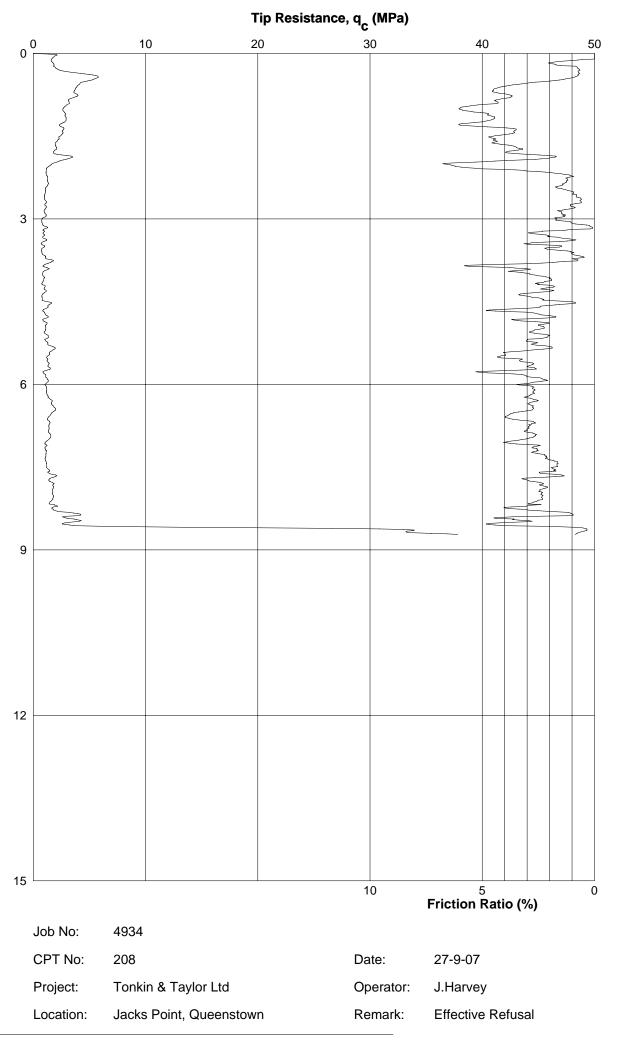




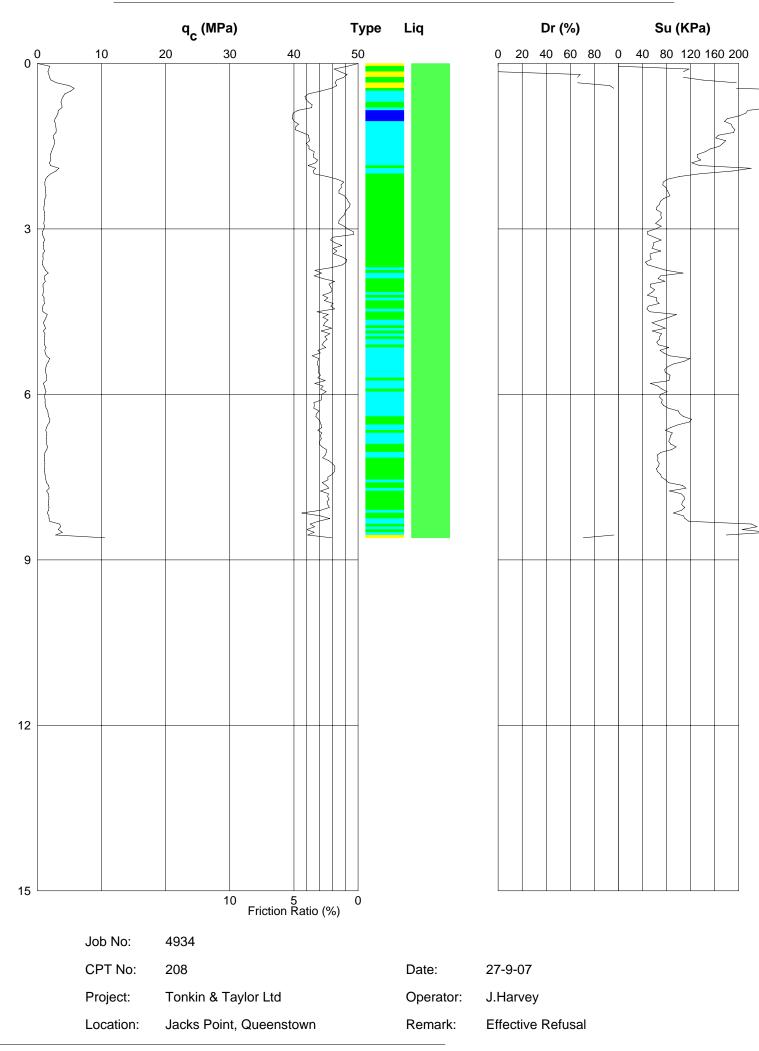




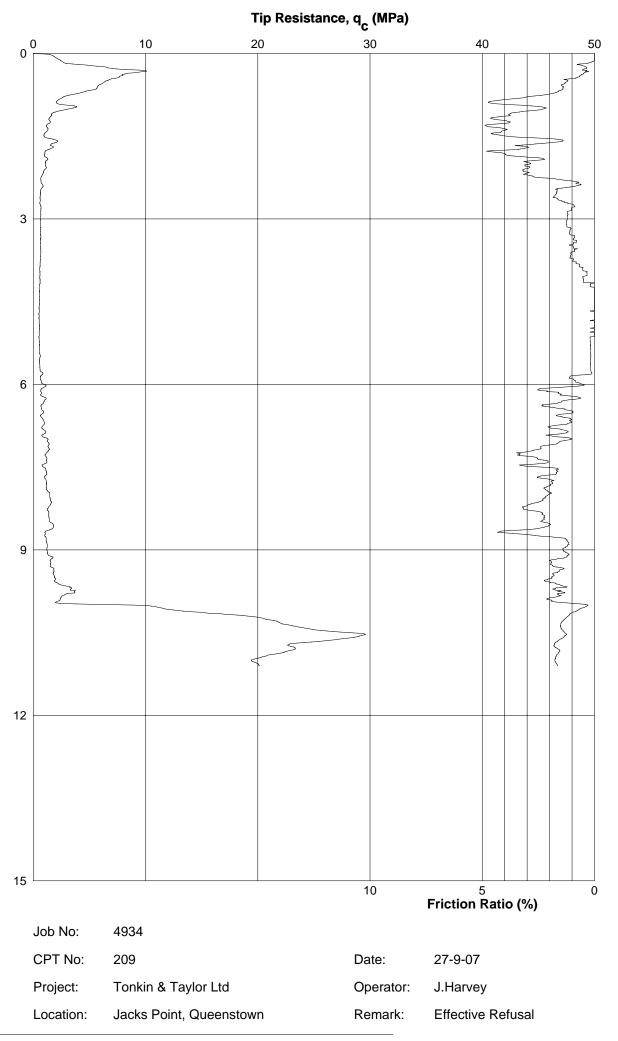


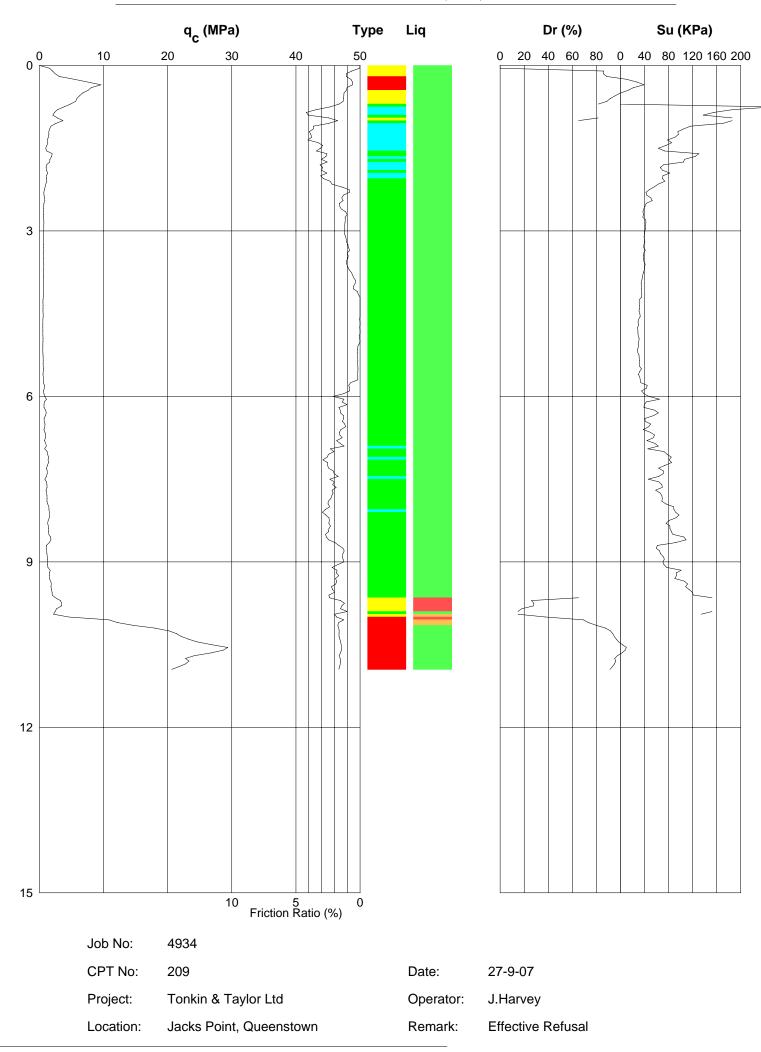


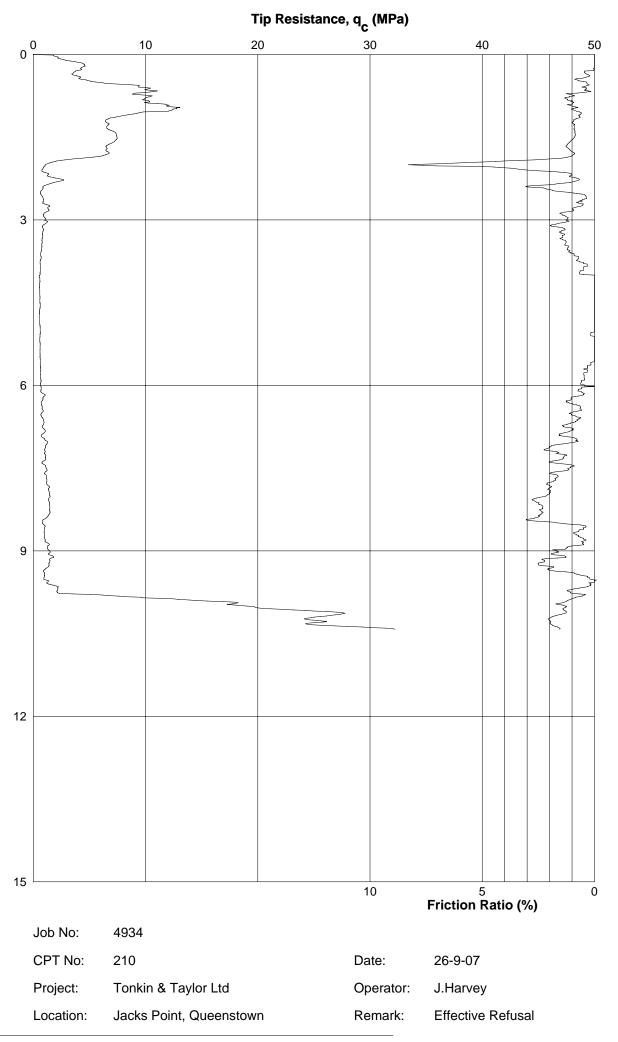
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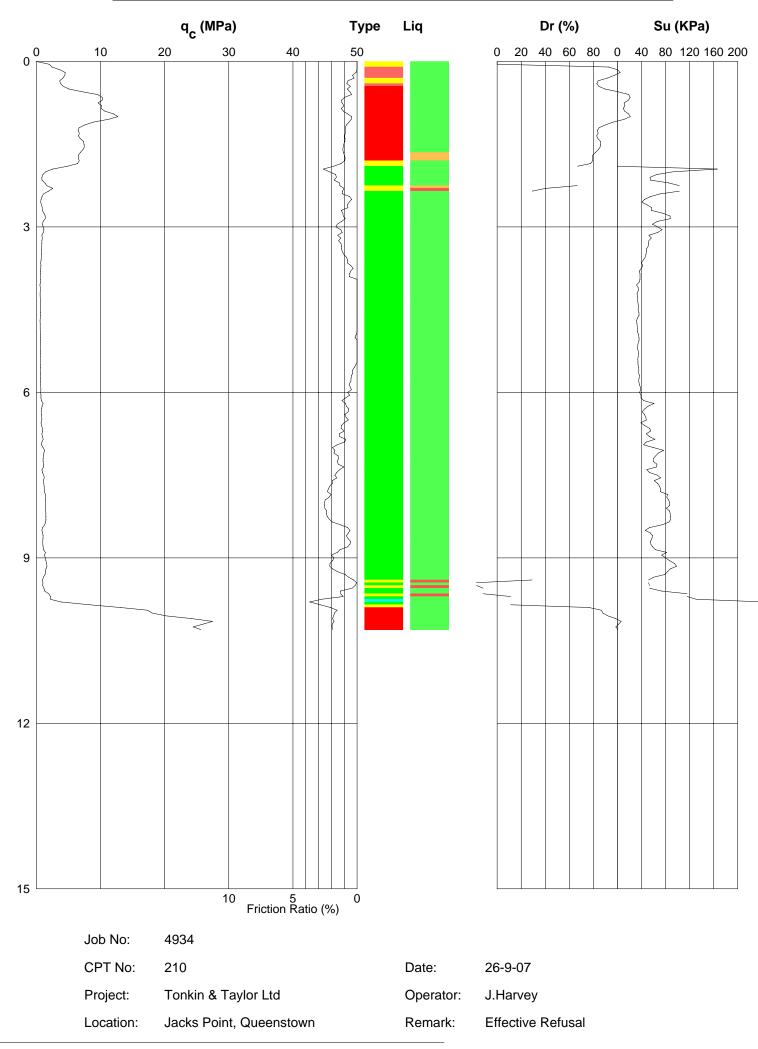


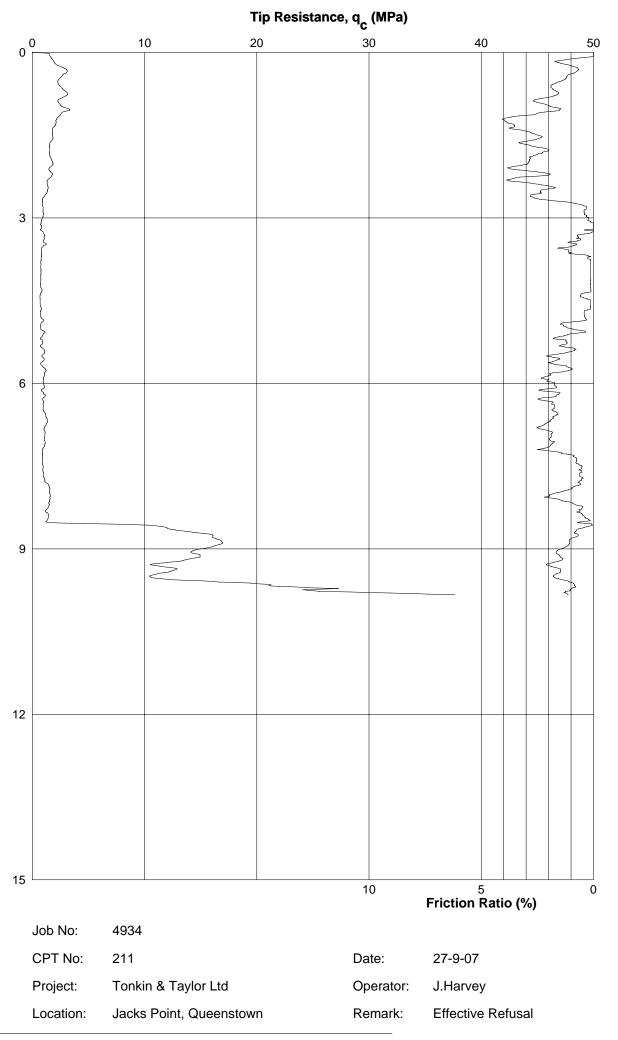
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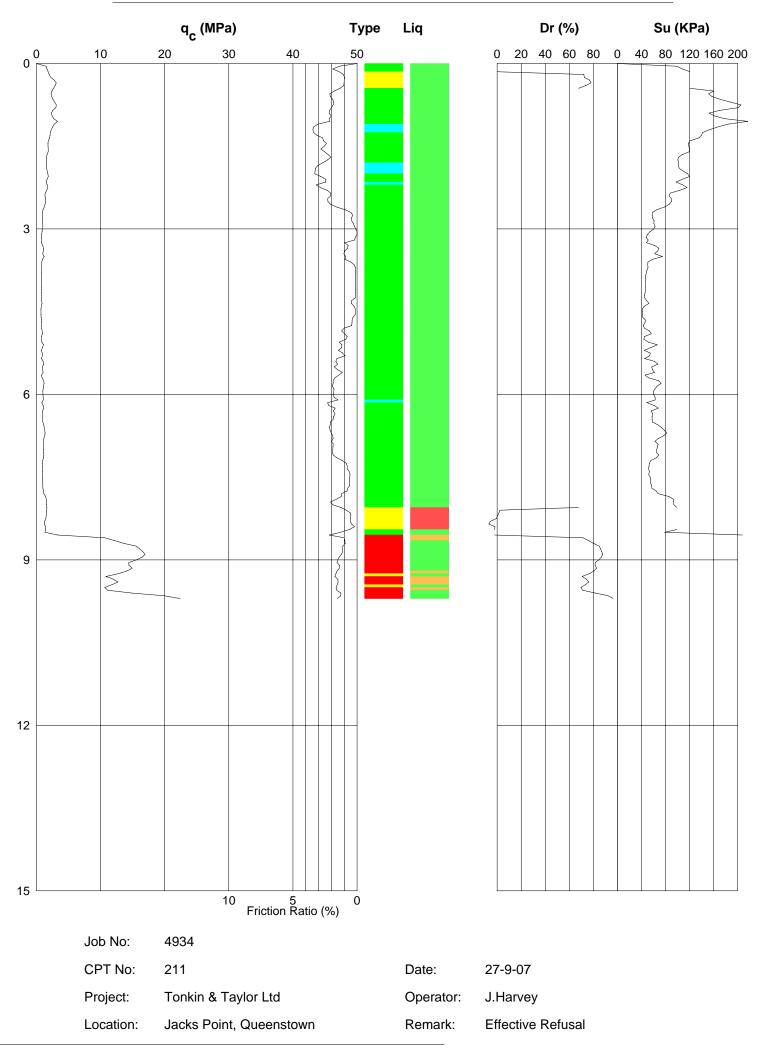




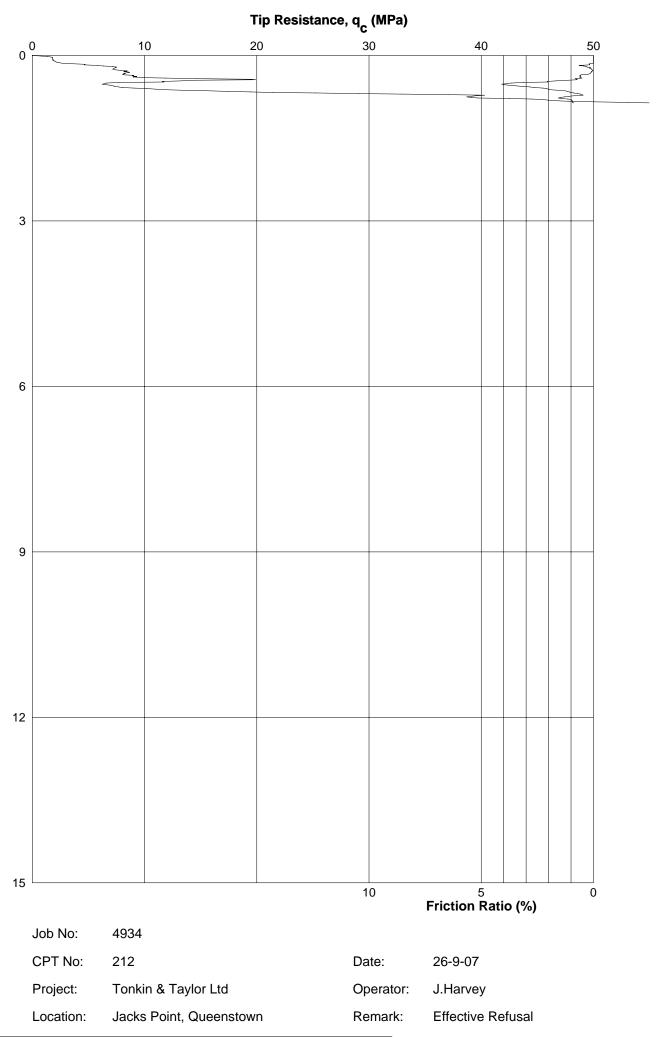


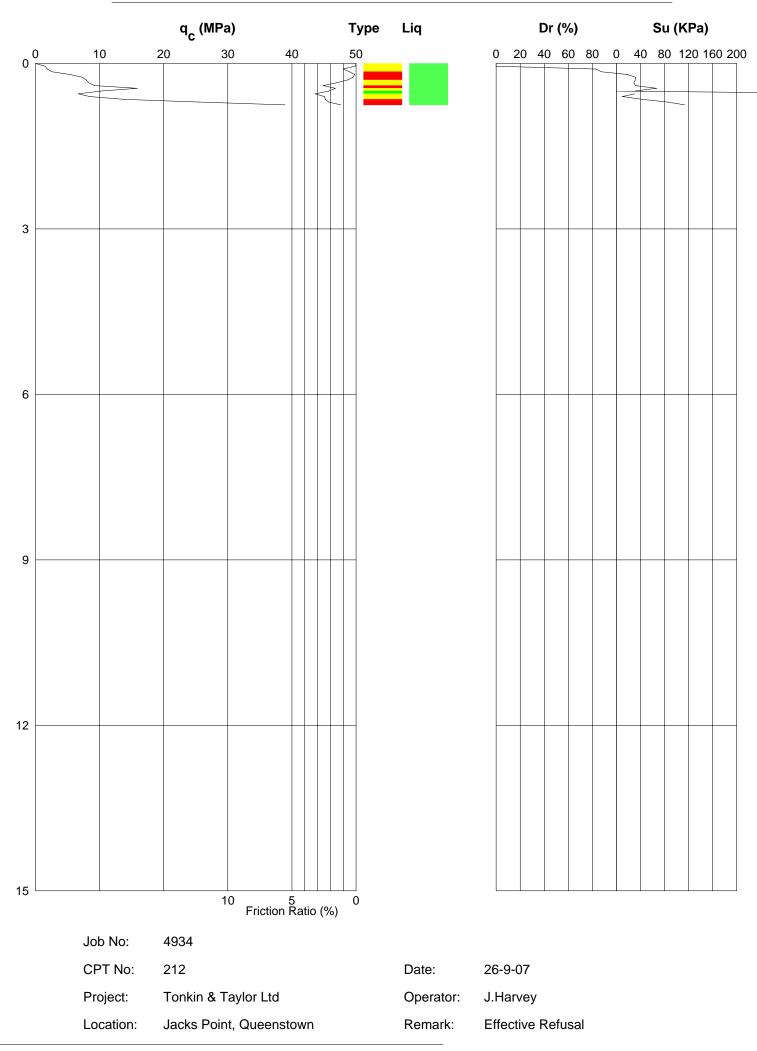


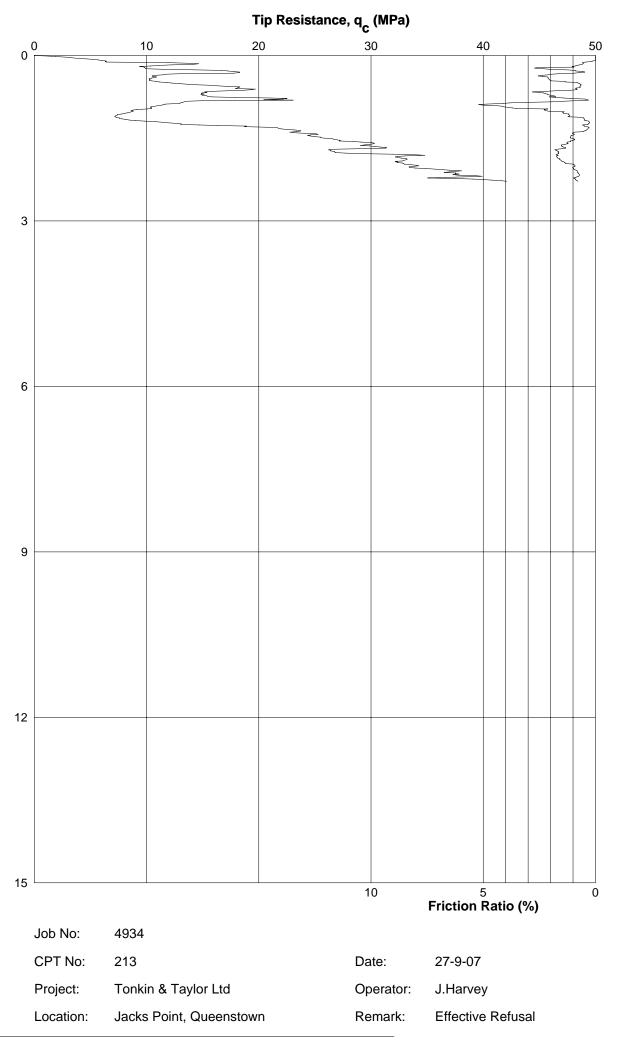
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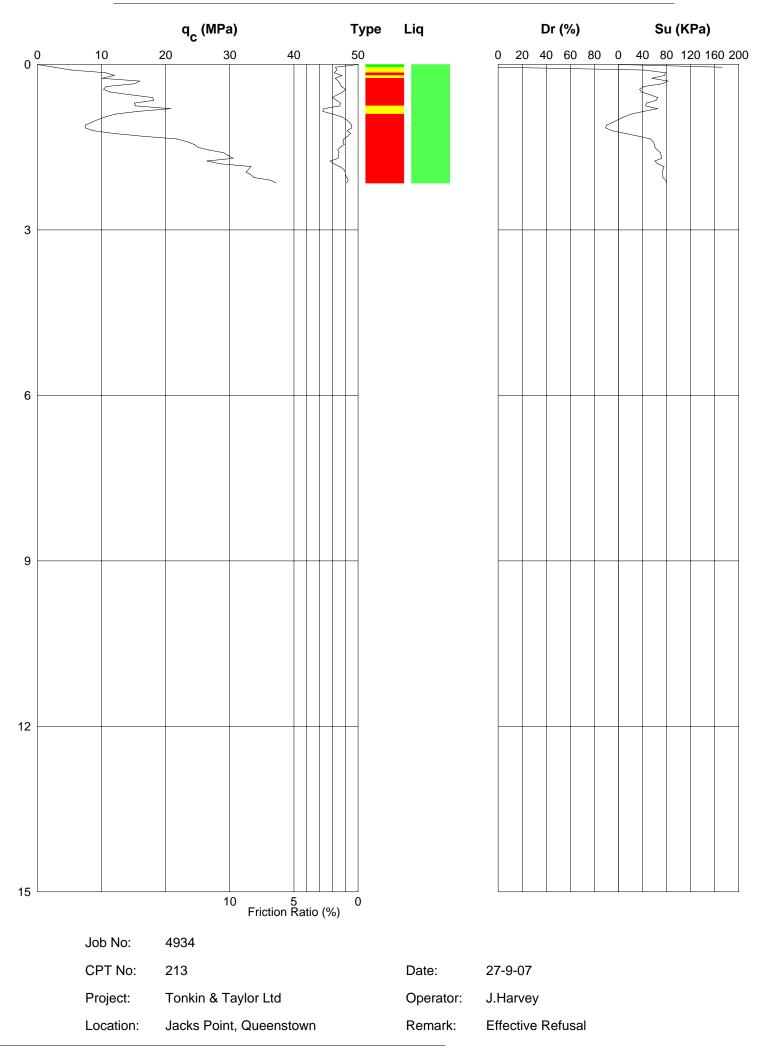


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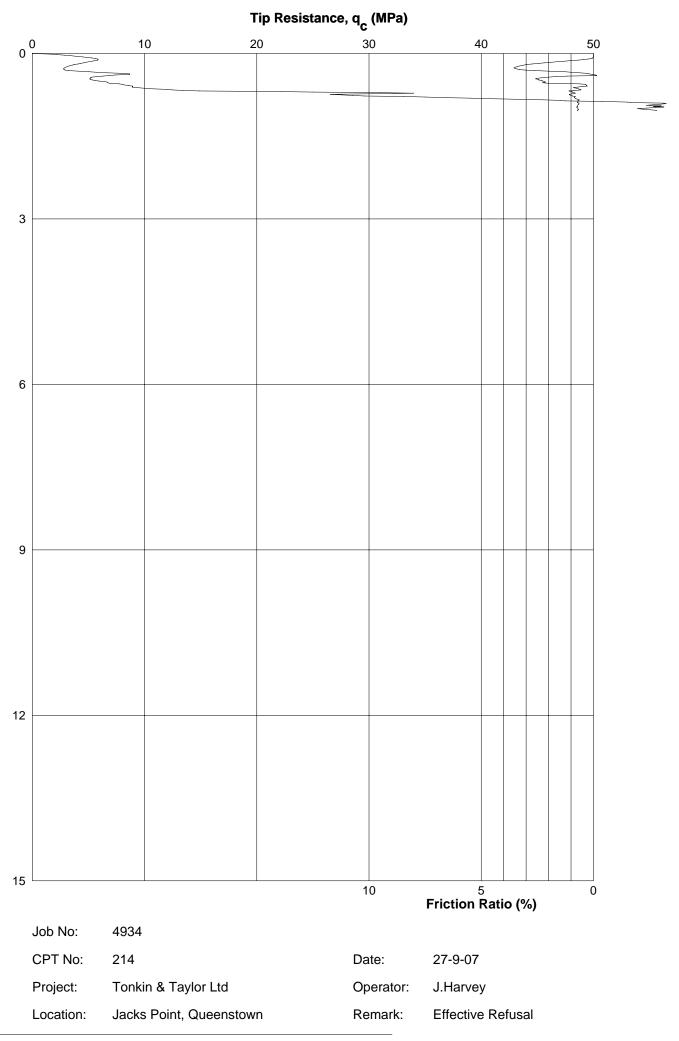


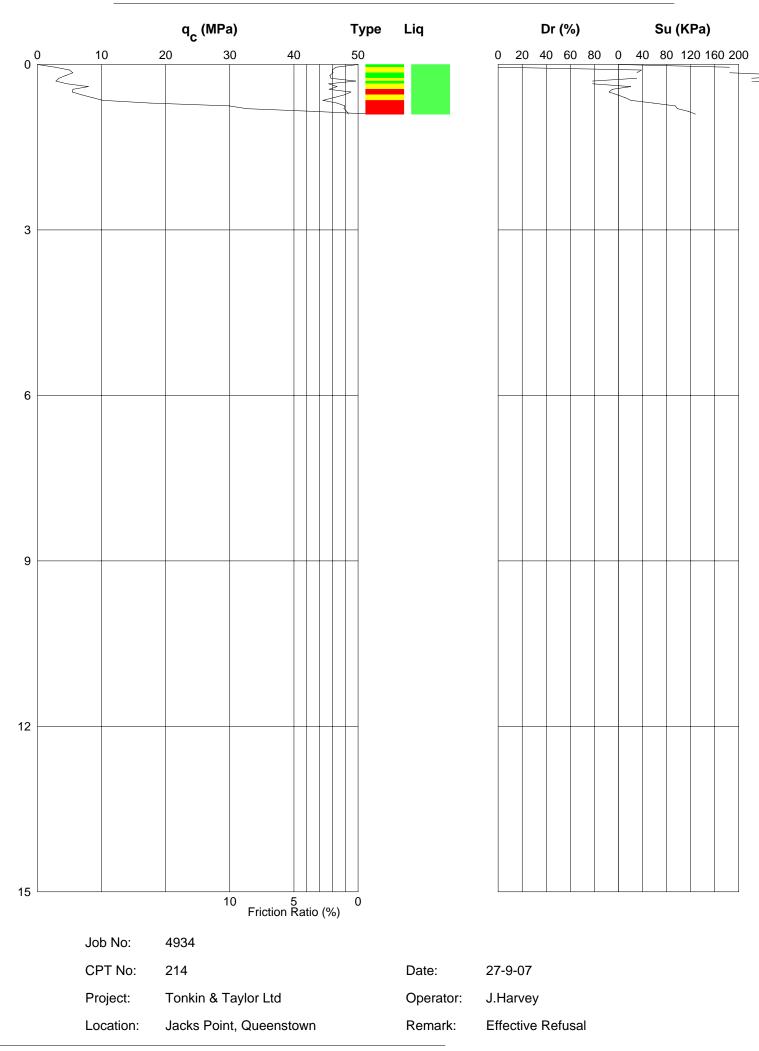


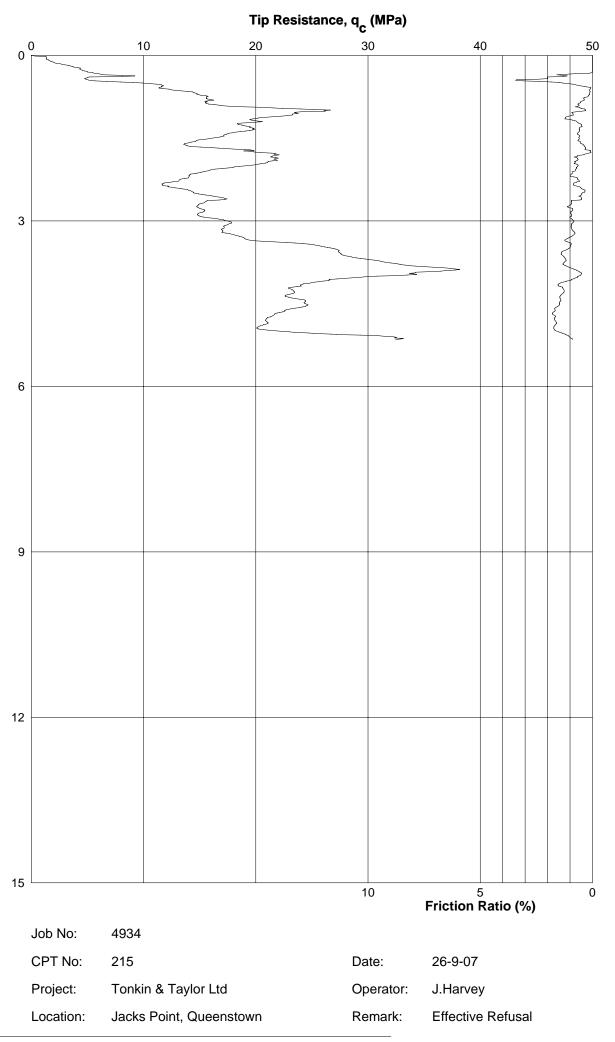




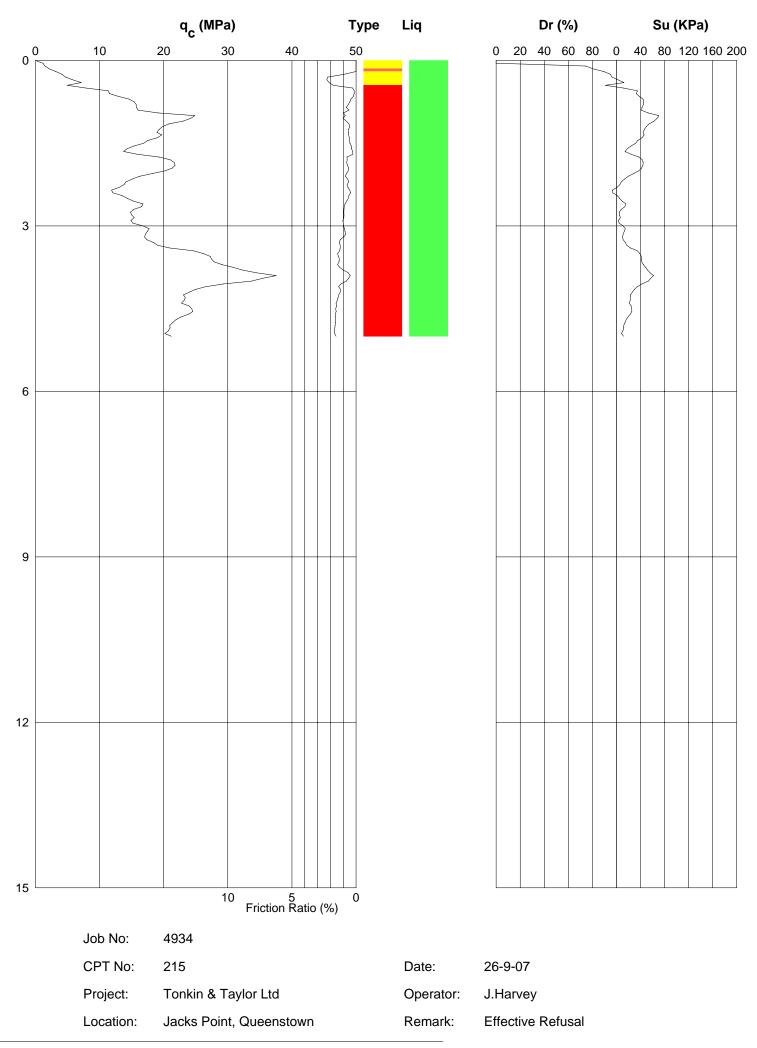
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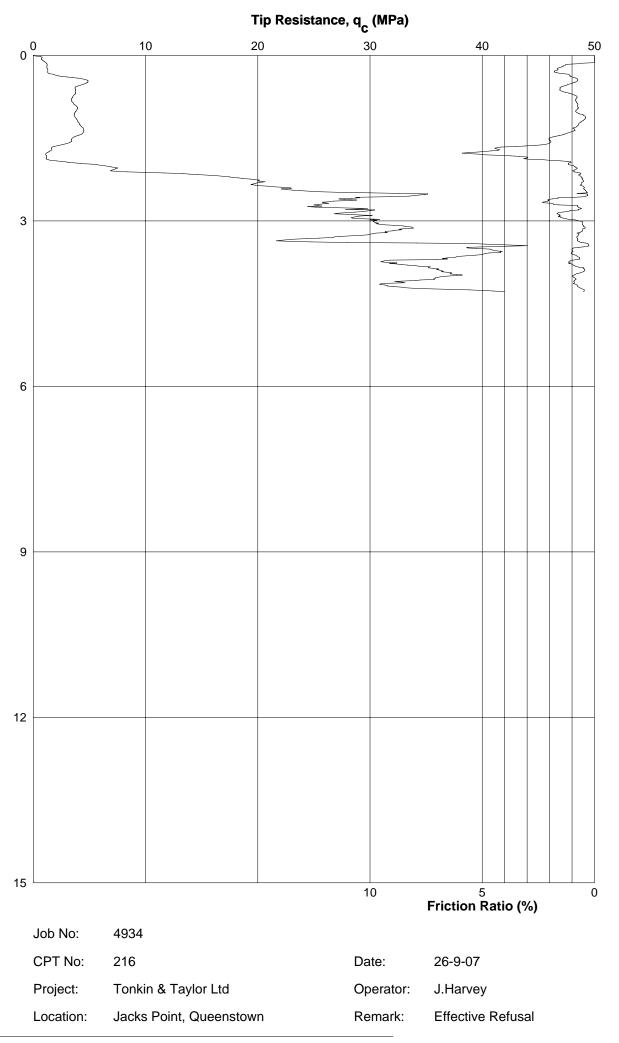




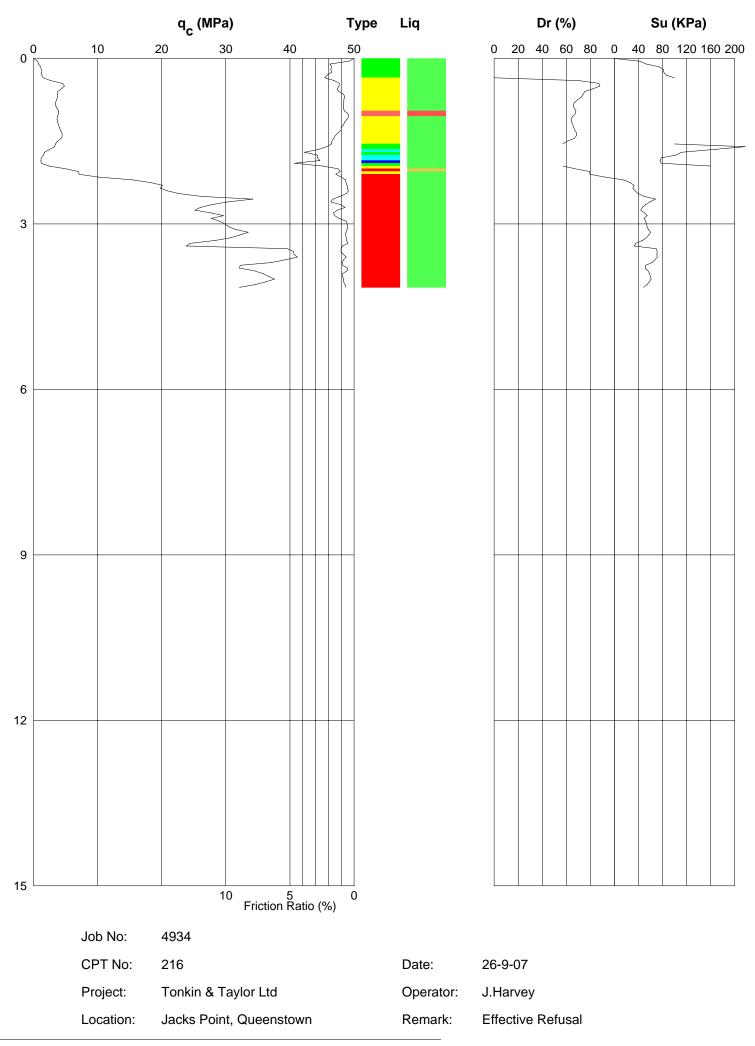


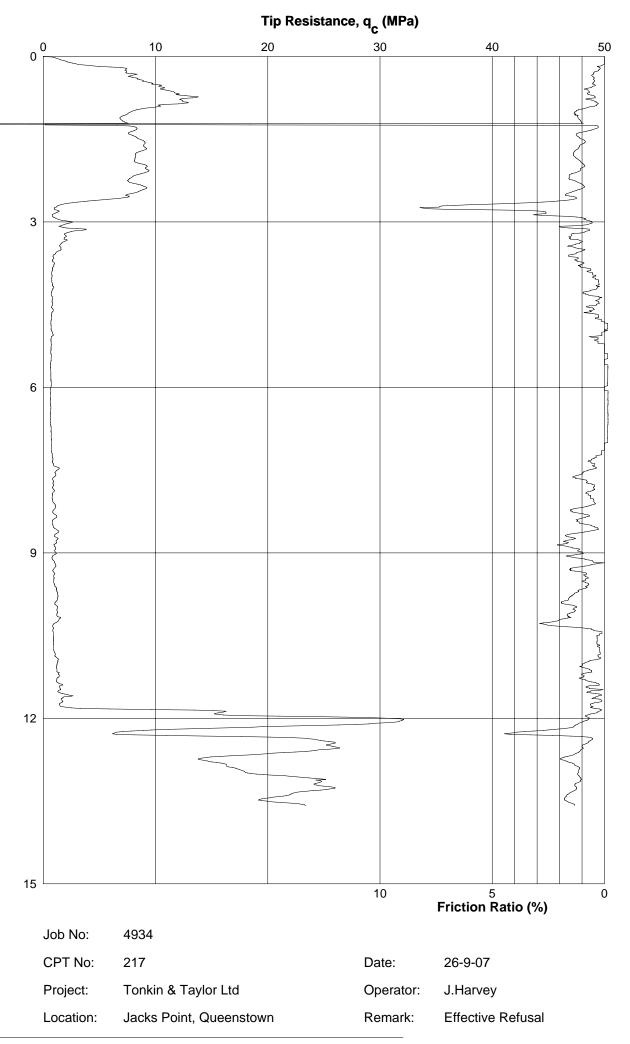
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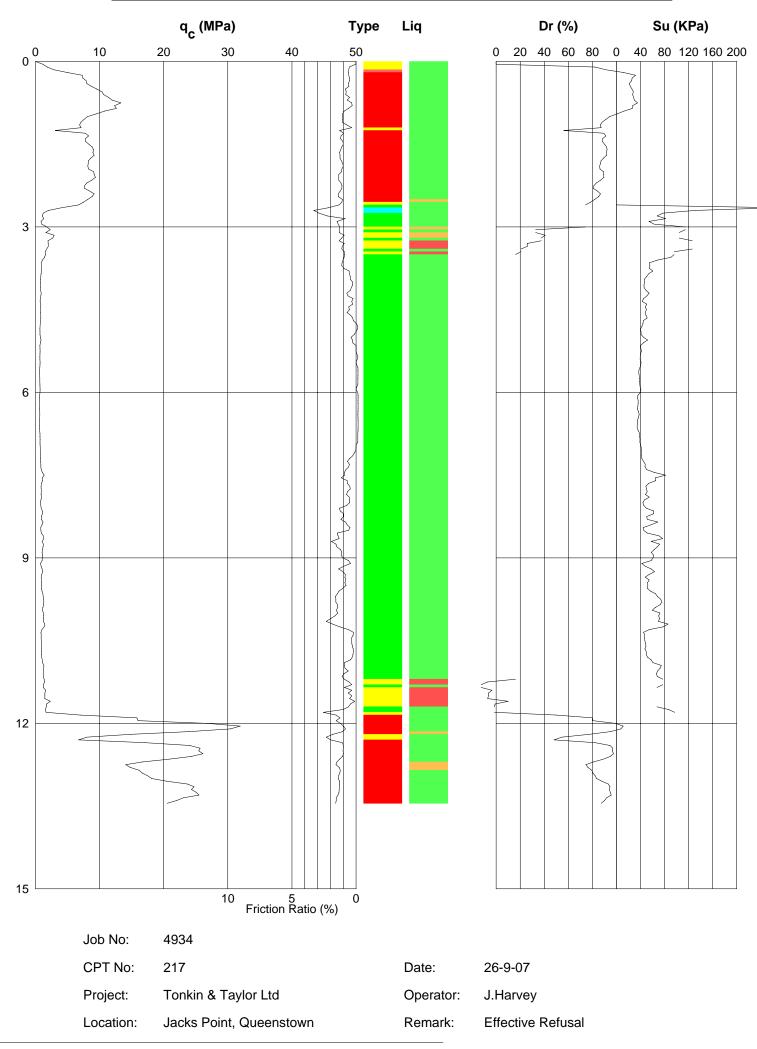


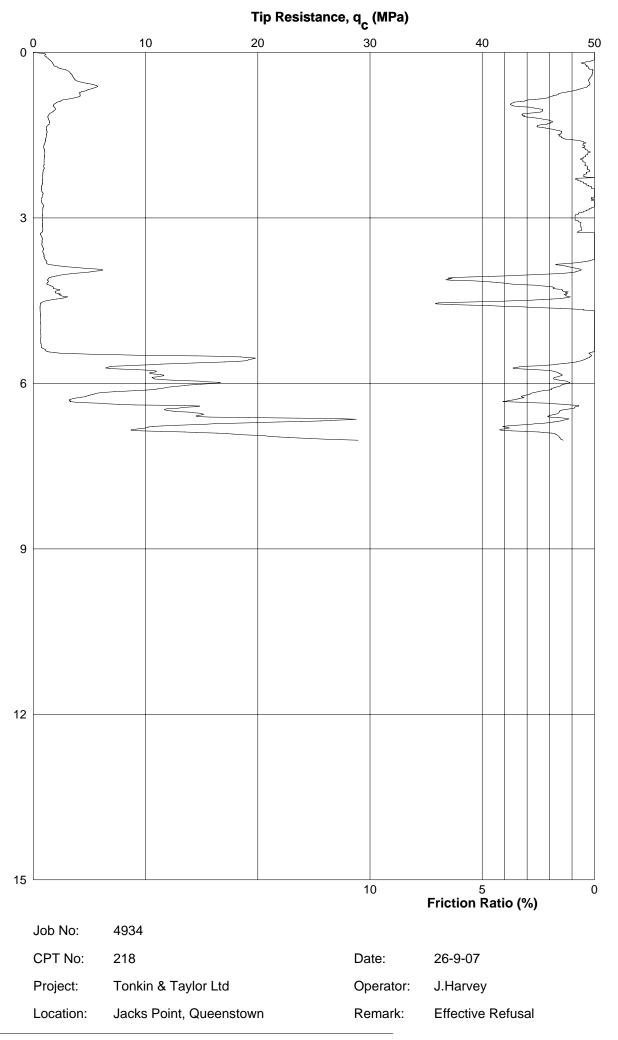
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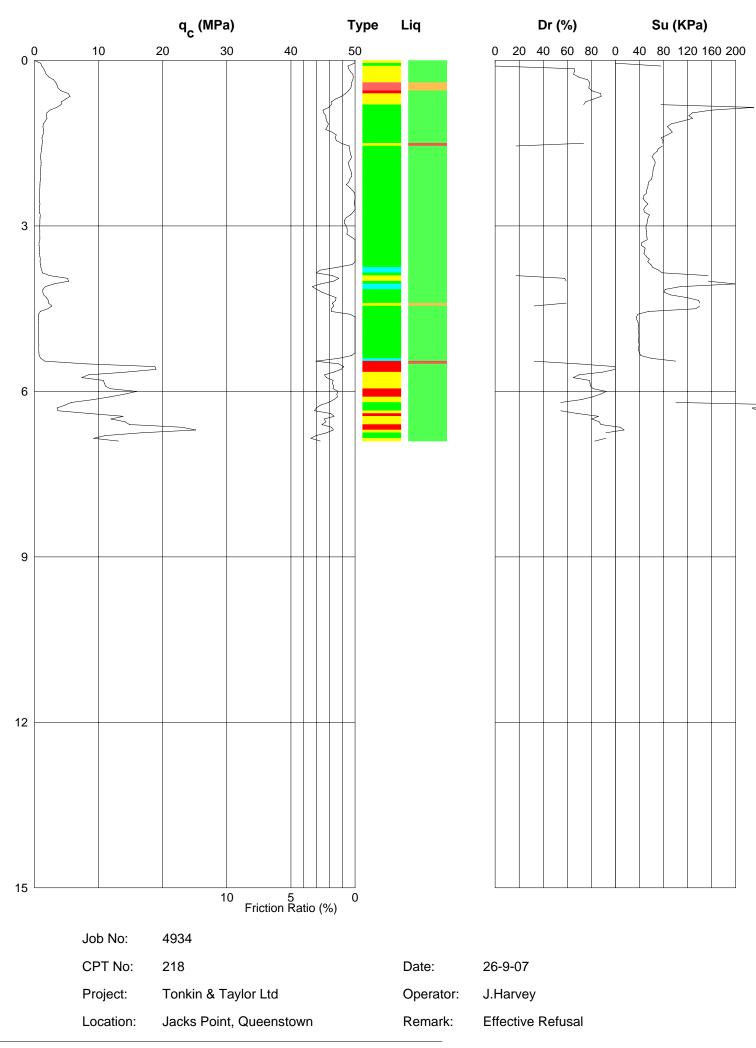


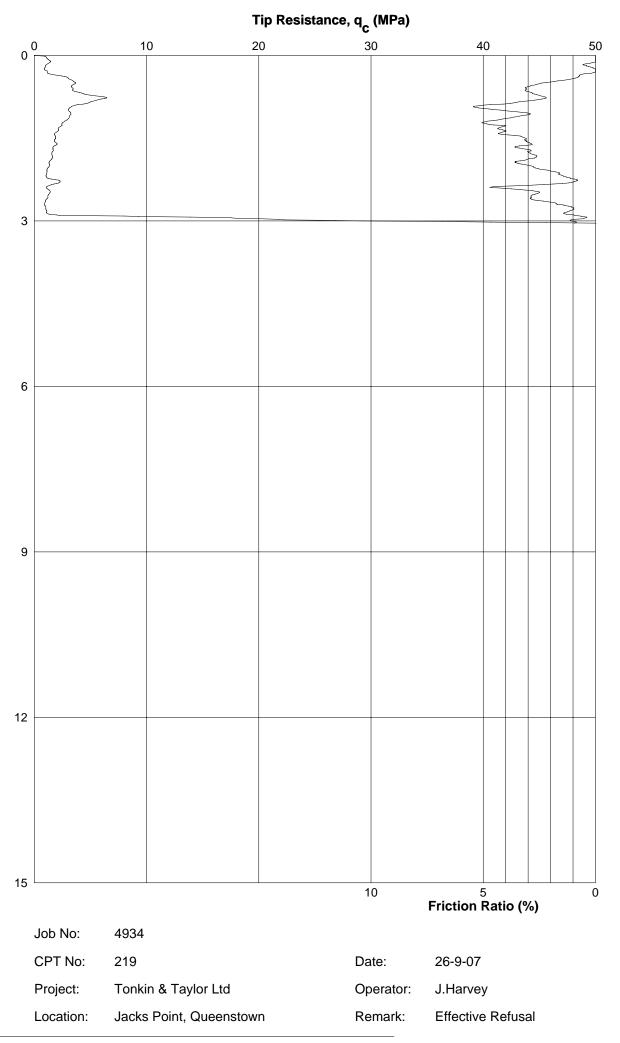


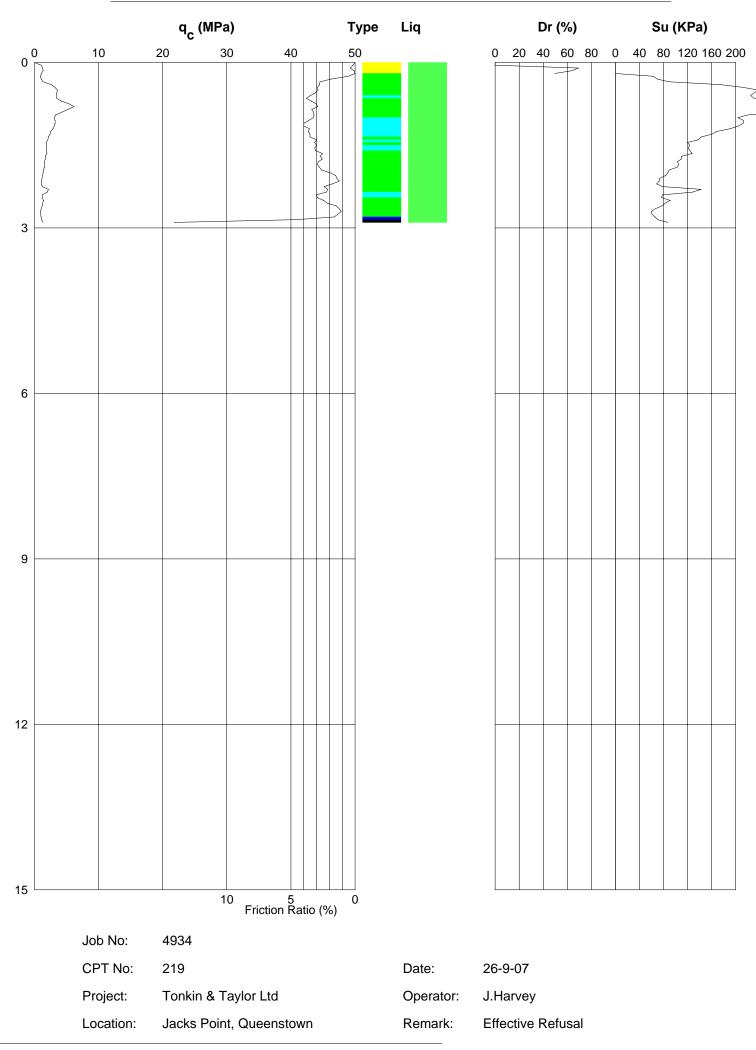
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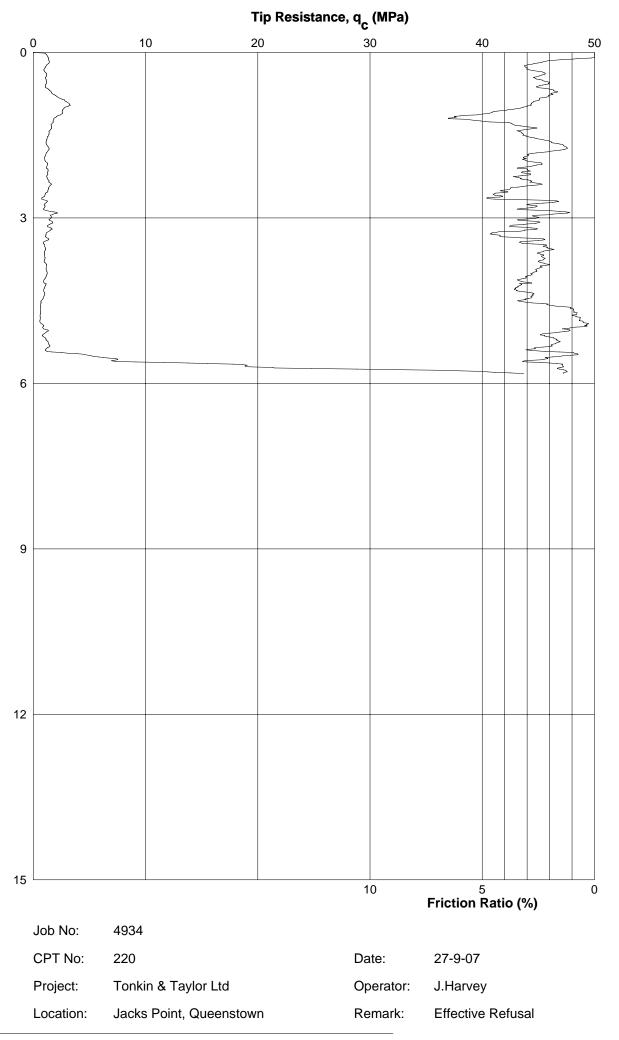




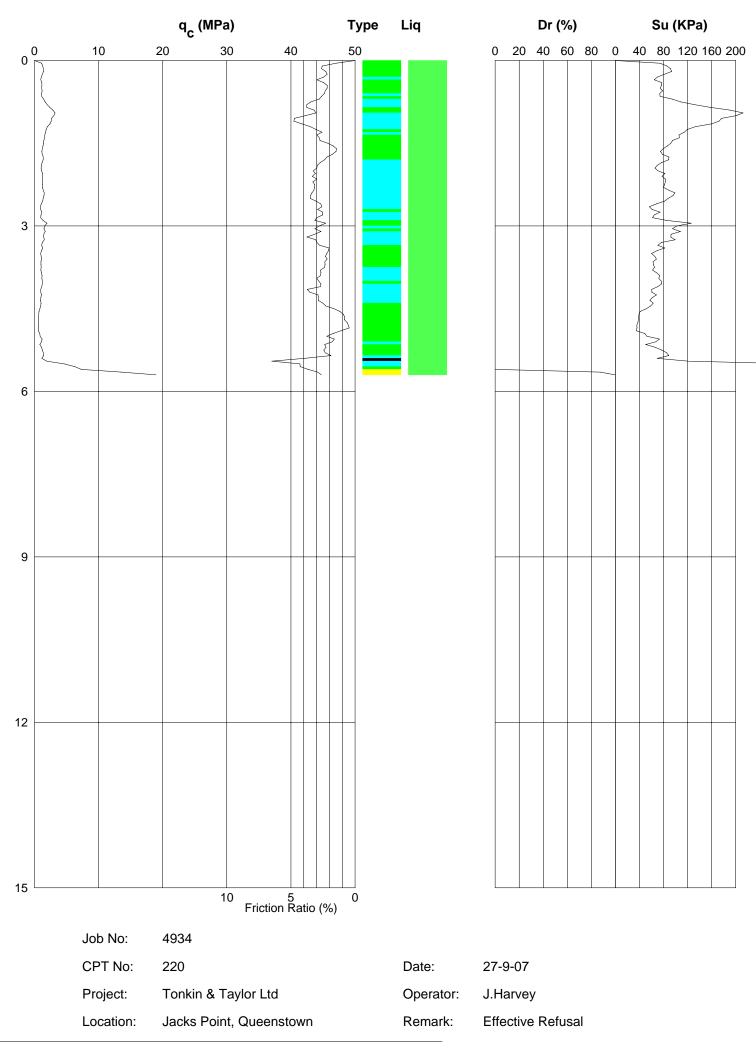


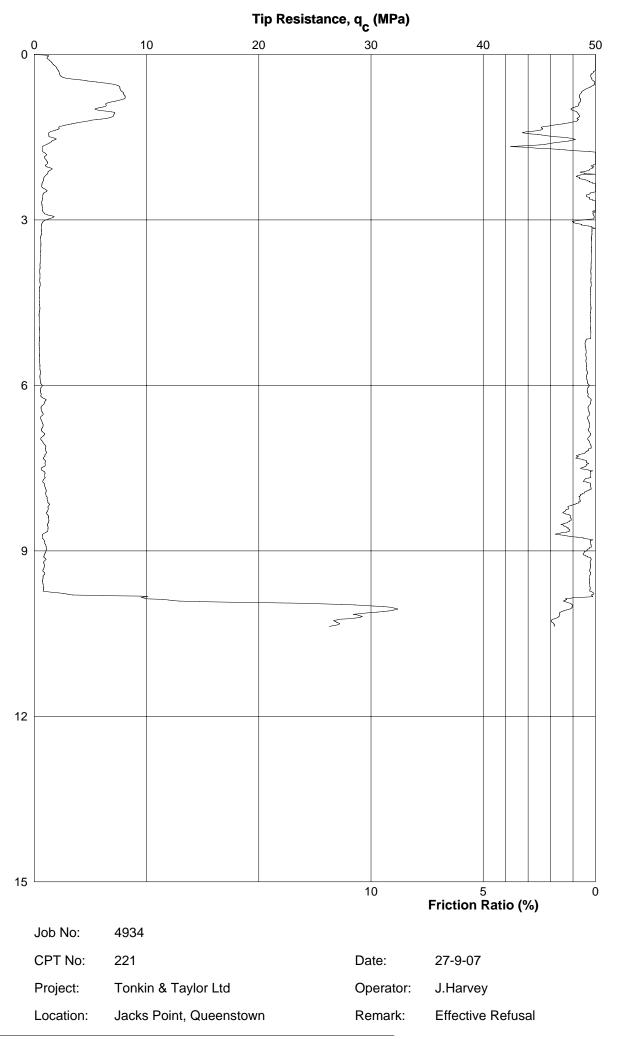




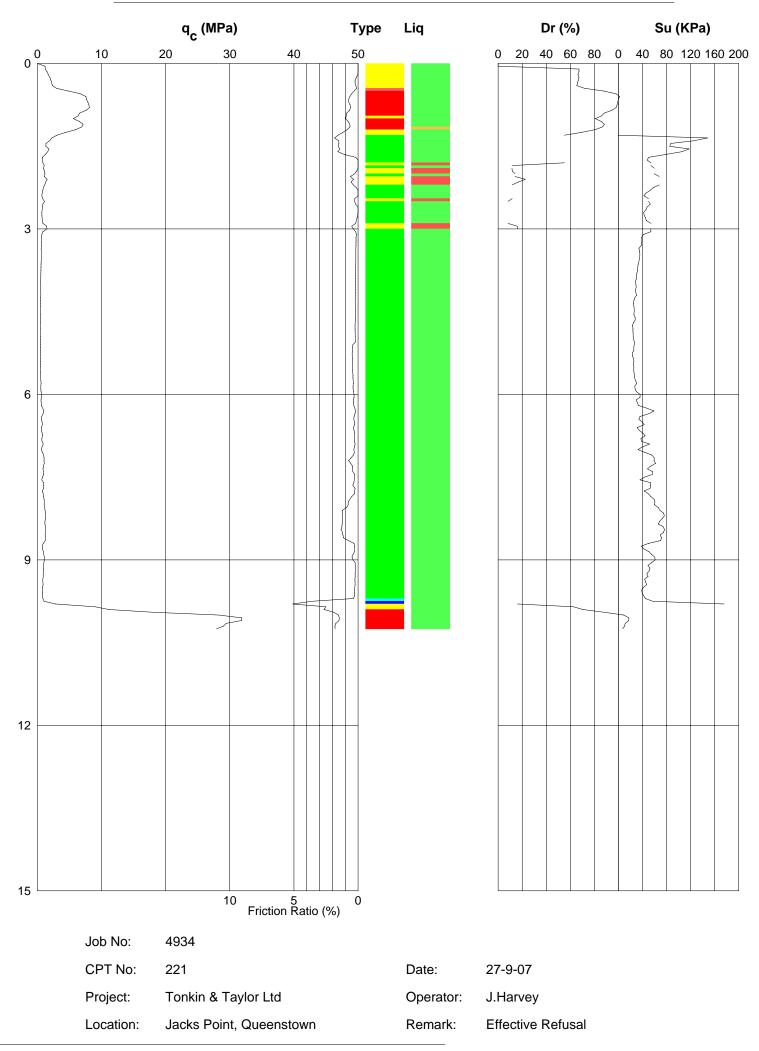


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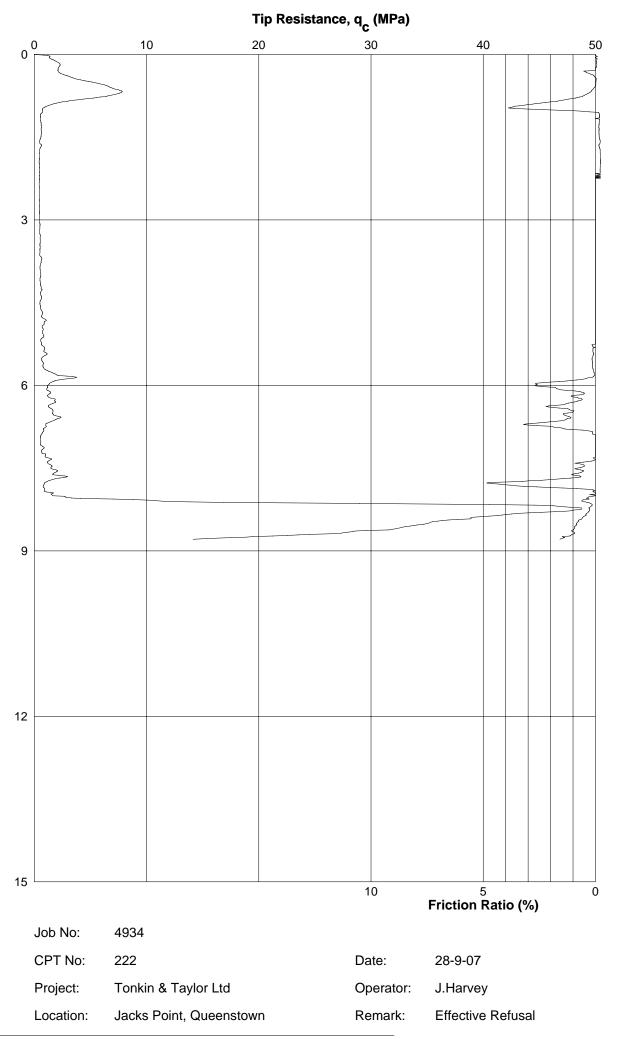




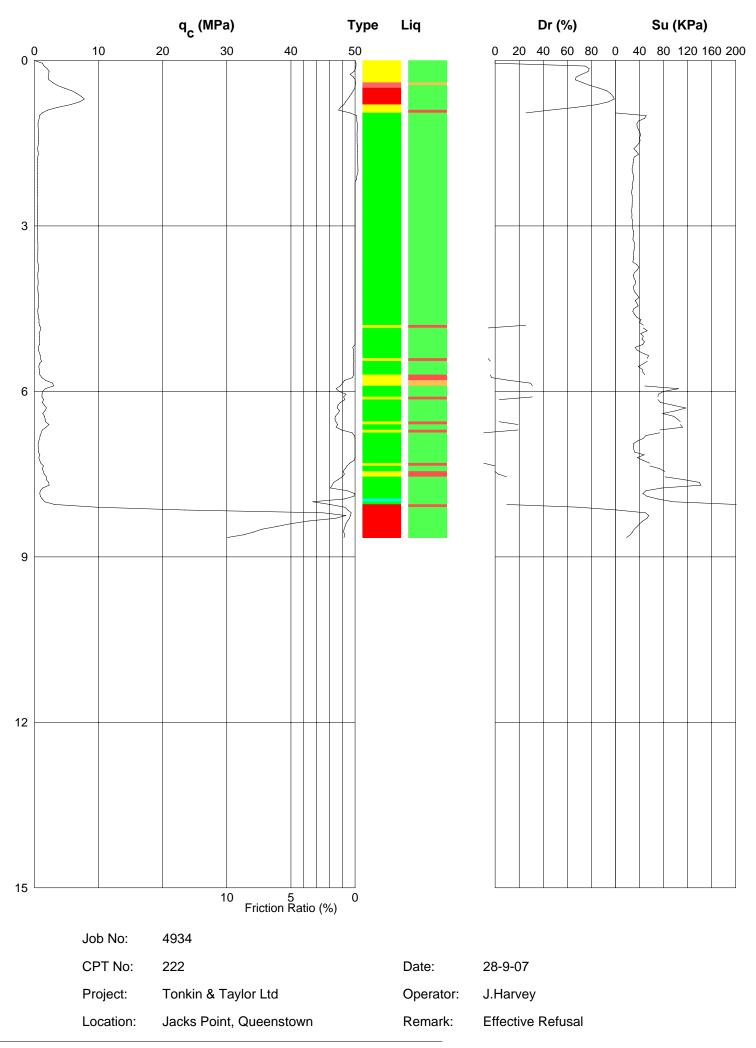
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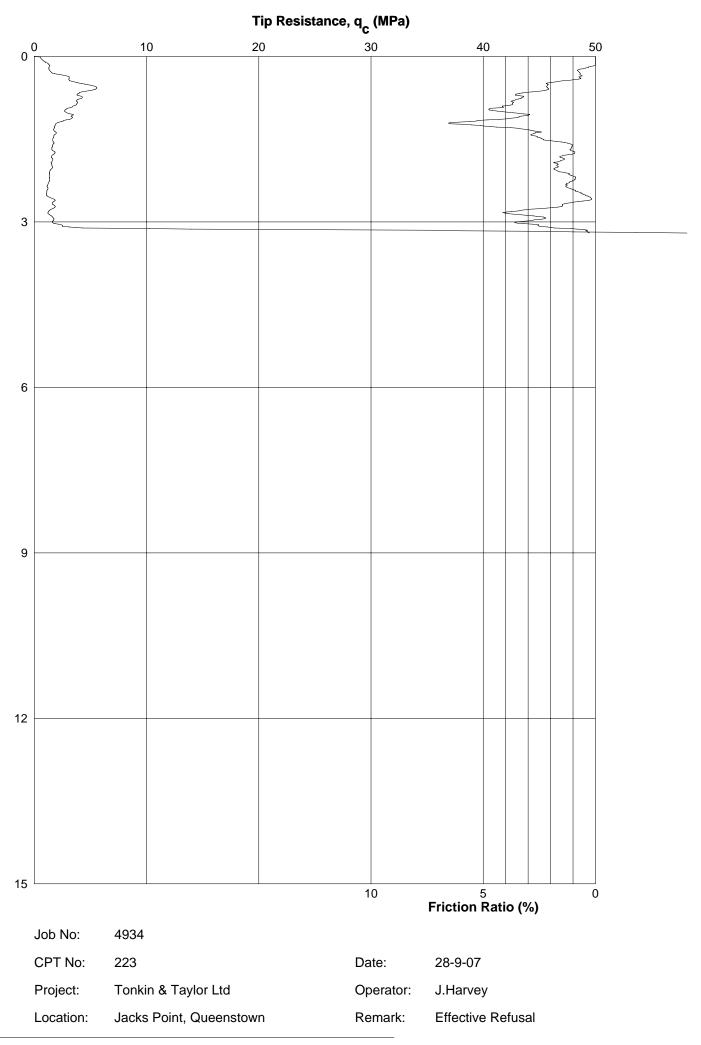


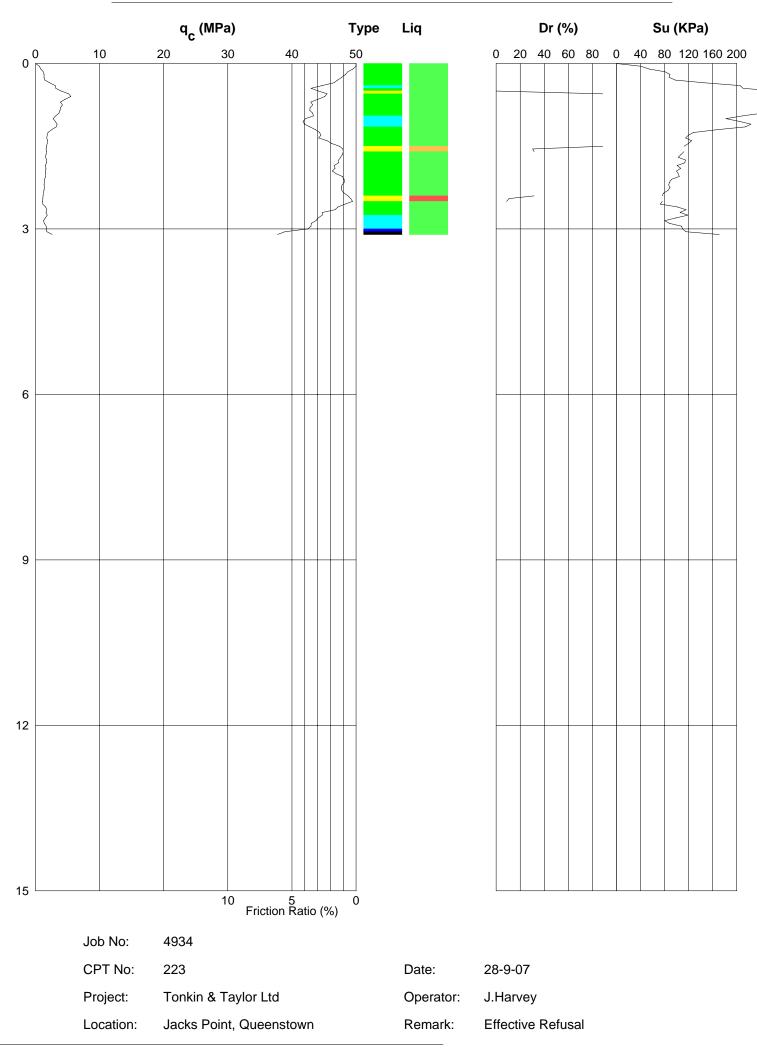
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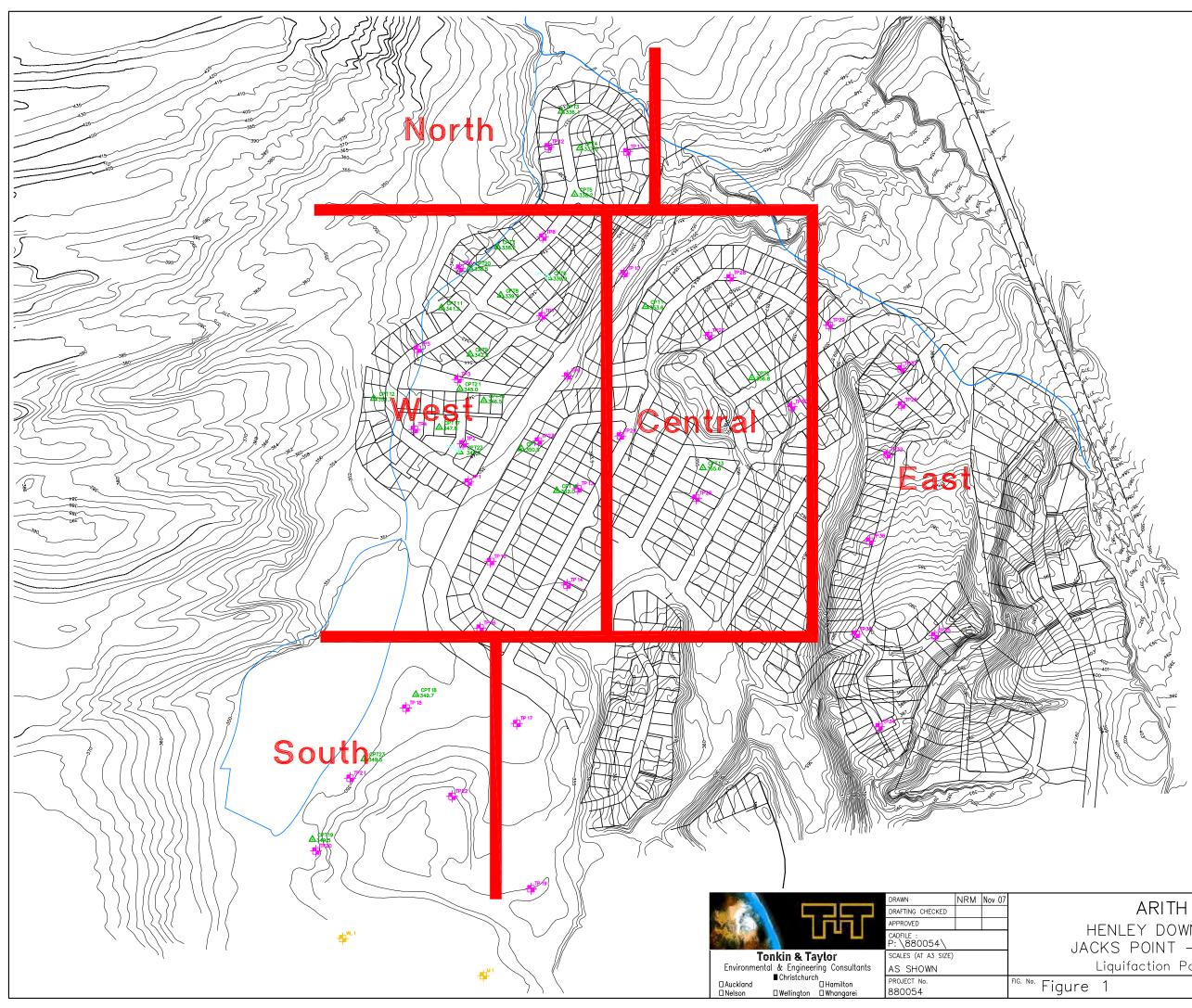
SITE INVESTIGATION







Appendix E: Liquefaction Site Plan



REV.

ARITH LIMITED HENLEY DOWNS SUBDIVISION JACKS POINT - HENLEY DOWNS Liquifaction Potential Site Plan

				•		
		SCALE	E 1:60	00		
ò	50	0 100	150	200	250 (n	n)

Predicted Settlement (mm)					
Location	SLS	ULS			
North	-	10			
West	30	80			
Central	-	5			
East	-	-			
South	40	65			



Appendix F: Liquefaction Analysis

